

Chehalis River Basin Flood Authority

Chehalis River Flood Water Retention Project Phase IIB Feasibility Study Draft Submitted to Chehalis River Basin Flood Authority for Review

**Original Draft Submitted November 10, 2010
Revised and Final Submitted April 14, 2011**

Prepared by:



570 Kirkland Way, Suite 200
Kirkland, Washington 98033

A registered professional engineering corporation with offices in
Kirkland, WA; Bellingham, WA; and Portland, OR

Telephone: (425) 889-2700 Facsimile: (425) 889-2725



April 14, 2011

Mr. Bruce Mackey
ESA Adolfson
1222 State Avenue, NE
Suite 202
Olympia, Washington 98506

SUBJECT: Chehalis River Flood Water Retention Project - Phase IIB Feasibility Study

Dear Mr. Mackey:

Please find attached a revised and final report for the Chehalis River Flood Water Retention Project Phase IIB Feasibility Study prepared by EES Consulting. We would like to acknowledge and thank you for your contribution to this study. In addition, we would like to recognize Shannon & Wilson, Inc. for their review of engineering costs and design concepts, Northwest Hydraulic Consultants for contributing flood depth data, and the U.S. Army Corps of Engineers for engaging with us on the economic methodology. Finally, we appreciate all the comments we received after we circulated the original draft.

Please contact me directly if there are any questions regarding this final report.

Very truly yours,

A handwritten signature in blue ink that reads "Gary S. Saleba".

Gary Saleba
President

cc: Dave Muller, Lewis County PUD

570 Kirkland Way, Suite 200
Kirkland, Washington 98033

Telephone: 425 889-2700 Facsimile: 425 889-2725

A registered professional engineering corporation with offices in
Kirkland, WA; Portland, OR; and Bellingham, WA

Contents

Executive Summary	1
Introduction and Scope	7
Report Organization	11
Hydrology Data and Historic Flows	12
Project Costs	26
Benefit-Cost Analysis Methodology	30
Benefits and Costs Estimated Using Corps Methodology	35
National Economic Development Benefit-Cost Analysis	60
Alternative Analysis	63
Regional Analysis	72
Benefit-Cost Analysis	77
Conclusions and Recommendations	79
References	81
Appendix A	83
Appendix B	84

List of Tables

ES-1	Engineering Analysis Results	3
ES-2	Benefit-Cost Ratios, 50-Year Period, 2010 Dollars.....	5
ES-3	Benefit-Cost Ratios, 50-Year Period, 2010 Dollars.....	6
1	Chehalis River near Doty Historic Crests.....	13
2	Comparison of Project Data.....	25
3	Project Cost Estimate.....	27
4	O&M Cost Estimate.....	28
5	Project Cost Summary (Base).....	29
6	HAZUS Damage Functions—Structure Damage	37
7	HAZUS Damage Functions—Content Damage	38
8	HAZUS Output: Residential Building and Content Benefit Calculation	39
9	HAZUS Output: Commercial Structure, Content, and Inventory Damages.....	40
10	HAZUS Output: Industrial Structure, Content, and Inventory Damages	41
11	HAZUS Output: Other Structure, Content, and Inventory Damages	42
12	Crop Value.....	43
13	HAZUS Output: Flood Acres of Agriculture.....	43
14	Agriculture Crop Damage Estimates	43
15	HAZUS Output: Relocation (Temporary Relocation Assistance) and Emergency.....	44
16	Public Assistance Benefits.....	45
17	Debris Removal, Truckloads	46
18	Building Clean-Up Costs Summary.....	46
19	Agriculture Field Restoration and Clean-Up Costs	47
20	Clean-Up Costs	47
21	Interstate 5 Closure Assumptions for Damage Estimates.....	51
22	Daily Cost Due to I-5 Closure	51
23	Interstate 5 Closure by Flood Return Interval.....	53
24	Railway Closure Costs by Flood Return Interval	54
25	Upper Chehalis Energy Value	56
26	South Fork Chehalis Energy Value.....	57
27	Recreation Visits and Value.....	59
28	Recreation Costs	59

29	Avoided Damages.....	61
30	Multi-Purpose Project Benefits.....	61
31	NED Benefit-Cost Ratios.....	62
32	Ecosystem Values for the Chehalis River Basin.....	66
33	Annual Reservoir Value.....	67
34	Upper Chehalis Wetland Added Annual Benefits.....	70
35	South Fork Chehalis Wetland Added Annual Benefit.....	70
36	Alternative Benefit-Cost Analysis Project Benefits.....	71
37	Alternative Benefit-Cost Ratios.....	71
38	Now-Dry Buildings in with Project Cases.....	72
39	Annual Flood Insurance Premiums.....	73
40	Total Property Value Increase.....	73
41	HAZUS Output: Regional Economic Loss.....	74
42	Households Affected.....	75
43	Intrinsic Value, Year 1.....	75
44	Regional Benefits.....	76
45	Regional Benefit-Cost Ratios.....	76
46	Benefit-Cost Ratios, 50-Year Period.....	77
47	Benefit-Cost Ratios with Low, Base, and High Construction Costs.....	78

List of Figures

1	Upper Chehalis near Doty-Long Term Daily Average Flows.....	13
2	Chehalis River near Doty Historic Gage Height.....	14
3	South Fork Chehalis near Wildwood-Daily Average Flows 1999 to 2005	15
4	Project Map Location.....	17
5	Map of Proposed Flood Water Retention Sites.....	18
6	Freeboard Concept.....	19
7	Structure Height and Flood Storage.....	20
8	Multi-Purpose Structure Height.....	22
9	Example Damage Curve	31
10	Example of Reduction in Damage Curve	31
11	Flood Elevations and Levee Elevations near Chehalis Airport	49
12	I-5 Truck Detour Routes	52
13	State Park Annual Visitation, Lewis County	58
14	Example of Reduction in Damage Curve	60
15	Reservoir Location and Forest Successional Stages	65
16	Flood Reduction Only Project Benefit-Cost Ratio with Low, Base and High Construction Costs.....	79

Executive Summary

Introduction

This Phase IIB Feasibility Study is part of the second of several phases initiated by the Chehalis River Basin Flood Authority (Flood Authority) to explore the option of flood reduction structures on the Chehalis River. The purpose of this study is to analyze the cost-effectiveness of the proposed projects using methodology used by and acceptable to the U.S. Army Corps of Engineers. Results of this study can be used to determine if a more detailed study of the benefits and costs is warranted in the future.

The Lewis County Public Utility District contracted EES Consulting, Inc. (EESC) to analyze whether flood retention structures in the Chehalis River Basin might be part of a solution to basin-wide flooding following the severe flood in 2007. In the initial scope (Phase I) EESC reviewed the possible benefits of developing water retention facilities, or flood storage structures, in the upper Chehalis River Basin.

After reviewing several sites, EESC identified and reviewed two locations at a level of detail consistent with an initial study. One site is located upstream of Pe Ell on the Upper Chehalis River, the other is on the South Fork of the Chehalis River. Total flood storage assumed for both sites was approximately 100,000 ac-ft. Flood water retention was the primary purpose, with instream flow augmentation secondary, and hydropower an ancillary benefit. The Phase I study, which examined potential costs and benefits, preliminarily showed that multi-purpose retention structures could be a cost-effective means to reduce flooding in the Chehalis River Basin. Following the release of the Phase I report, EESC received important feedback about this initial study.

The Flood Authority subsequently contracted for additional work in Phase II. The original scope of work for Phase II was split into Phase IIA, and Phase IIB. Phase IIA included a geology and geotechnical study of the potential sites; this study concluded that no major impediments exist to the construction of flood storage structures at either site. Phase IIA also included the development of an environmental scoping document describing future environmental studies related to the potential structures.

The Flood Authority then approved moving forward with Phase IIB to refine the basic engineering estimates developed during Phase I, and to update the economic information using the Corps of Engineers' methodology. During the Phase IIB process, the Authority asked what a single purpose flood water retention structure might look like, and whether it might be cost effective. Accordingly, this Phase IIB Feasibility Study examines both single purpose (flood only) and multi-purpose (flood, stream augmentation, and hydropower) structures. The Flood Authority also elected to defer work related to environmental issues. Instead, much of this work is currently underway by Anchor QEA as part of the fisheries studies; results are currently expected in June 2011.

Phase IIB Incorporates Comments and Feedback from Phase I

Feedback received about the economic and engineering analyses received during earlier studies has been incorporated into the Phase IIB report. Following the release of the initial report, Phase I, EESC received numerous comments and feedback, both written and by meeting with various stakeholders. Flood Authority Board members and the Washington Department of Ecology provided written feedback, while EESC met with the U.S. Army Corps of Engineers (“Corps”). Phase I work was based on flow data approximated from the USGS gage near Grand Mound. The Washington State Department of Fish and Wildlife¹ suggested that flow data from the Doty gage be used for the analysis. Analysis in the Phase IIB work was updated to incorporate data from the Doty gage for the period of record to March 2010, including the 2009 flood. Additional discussion on this gage data is included below.

In addition, the Washington Department of Transportation (WSDOT)² responded to the Phase I study with updated values for the avoided cost to raise Interstate 5 (I-5). In the event that the proposed projects keep I-5 from flooding in the Chehalis/Centralia area, WSDOT would not need to raise the freeway and would avoid a cost of \$100.5 million (2009 dollars). This report recognizes that the proposed structures and minor levee modifications would not prevent all flooding, but could help address the significant impacts of severe flooding.

Consultation with the Corps also resulted in changes to the analysis methodology and definition of possible project options. First, the benefit-cost analysis was updated to comply with the methodology used for such proposals by the Corps in the 1983 *Economics and Environmental Principles & Guidelines for Water and Related Land Resources Implementation Studies*³ and the Corps’ *Planning and Guidance Notebook*⁴, collectively referred to hereafter as the “*Principles & Guidelines*.” Second, rather than modeling the benefit for two flood events—the 2007 flood, and a hypothetical 100 year flood— as was done in the Phase I report, the Phase IIB analysis is based on a probability of exceedance damage curve for 10, 50, 100, and 500 year flood events. Finally, because the Corp first evaluates any project as a single-purpose project (in this case a flood reduction only project), and then evaluates the incremental costs and benefits for a multi-purpose project, Phase IIB also includes an evaluation of the flood-reduction only benefit-cost ratios.

¹ Beecher, Hal. Comments from the Washington Departments of Ecology and Fish and Wildlife on Chehalis River Retention Structures Scoping Document and Proposed Studies. Enclosed in the Department of Ecology letter to the Flood Authority Subcommittee. January 7, 2009.

² Gernhart, Bart S. WSDOT letter to Dave Muller, Manager Lewis PUD. March 9, 2009.

³ U.S. Water Resources Council, March 10, 1983. *Economic and Environmental Principles and Guidelines for Water and Related Land Resources Implementation Studies*.

⁴ U.S. Army Corps of Engineers, April 22, 2000. *Planning Guidance Notebook*. Department of the Army. Washington D.C.

Comments related to the potential environmental and fisheries impacts have been given to Anchor QEA as part of the on-going Chehalis River fish study.

Assumptions Incorporated into Phase IIB

There are several basic assumptions used throughout the Phase IIB study, including the following:

- The development of any large scale project is an iterative process. While the Phase IIB study provides additional information, it also indicates the needs for additional study and design work. Cost estimates will be updated as more detailed information is available.
- The Phase IIB Study focuses on using the U.S. Army Corps of Engineers methodology. This methodology generally does not monetize environmental benefits or costs.
- Because hydraulic modeling is not available downstream of the gage at Grand Mound (RM 59.9 near the Lewis/Thurston County border), this analysis does not include costs and benefits to Thurston and Grays Harbor County.
- Because there is an on-going study by Anchor QEA about potential impacts to fisheries, the figures included for fish mitigation are provided as a place holder. The results of these studies, plus potential opportunities for mitigation, will need to be examined further.
- Detailed environmental work through the NEPA/SEPA permitting process will be necessary to build this kind of project; this will provide significantly more detailed information.

Phase IIB Study Results

EESC engineers analyzed topographical information and flow data from the Doty gage to estimate costs for two types of structures for each site. The initial Phase IIB scope included preliminary design and cost estimates for multi-purpose structures (including flood water retention, and storage for summer flow augmentation and hydropower). After consultation with the Corps, the Flood Authority asked EESC to also develop cost estimates for structures designed for flood retention only. All cost estimates are based on the assumption that these structures would be earthfill structures. The two types of structures for each site are summarized in Table ES-1.

Table ES-1
Engineering Analysis Results

	Upper Chehalis		South Fork	
	Flood Reduction	Multi-Purpose	Flood Reduction	Multi-Purpose
Flood Storage, ac-ft	80,000	80,000	20,000	20,000
Flow Augmentation Hydropower Storage, ac-ft	NA	65,000	NA	13,500
Structural Height, ft	238	288	170	200
Construction Cost ⁵	\$165,230,000	\$245,060,000	\$93,060,000	\$148,540,000

The flood reduction structures are free-flowing and would operate such that natural flows are not affected except during and immediately following a flood event. A free-flowing structure, as defined in this report, is one that passes river flows without interrupting those flows or changing their timing or quantity, except for short periods during and subsequent flood events. The single-purpose structures are defined as free-flowing, since, under normal conditions, they do not interfere with natural river flows. When a flood event occurs, flood waters would be retained and stored until the event subsided and they could be safely released in a controlled manner.

The multi-purpose structures store water so that releases in the summer months are greater than natural flows. The multi-purpose structures have intake towers located in the reservoirs so that water can be released from varying depths, offering the potential to take advantage of the typical pattern in temperature-stratified lakes, where sub-surface waters are typically cooler than water in the uppermost layers. Accordingly, flow augmentation in the summer months may be managed to yield lower in-stream water temperatures in the reaches below the structure(s).

As a safety requirement, both the single-purpose and the multi-purpose structures have uncontrolled spillways so that the structures are not overtopped.

Phase IIB also included an update on the economic benefits, and calculation of the resulting benefit/cost analysis. This analysis incorporates substantial feedback received following the Phase I report, and is based on the Corps' *Principles & Guidelines* documents and the HAZUS model developed and used by the Federal Emergency Management Administration ("FEMA").

Three benefit-cost ratios are provided for each proposed water retention structure option. First, the National Benefit-Cost Analysis is provided. These benefit-cost ratios are developed using the *Principles & Guidelines* methodology used by the Corps.⁶ Second, an Alternative Benefit-Cost Analysis added monetized environmental benefits to each project. Finally, regional benefits

⁵ The costs in lists construction costs in 2010 dollars, while Table ES-2 lists 50 year total cost of the project on a NPV basis. Costs will be updated as further information is available.

⁶ The Corps' *Principles & Guidance* methodology does not consider social and environmental costs and benefits.

and costs were added for a Regional Benefit-Cost Analysis. The Regional Analysis also included the monetized environmental benefits.

The avoided costs of flooding for each structure were calculated by determining flood damages in the without-project and with-project cases. The Corps' 2003 report⁷ provided the basis for methodology and source for data used to calculate avoided costs, such as clean-up costs, damages to agriculture crops and avoided transportation costs. The expected annual avoided damage is calculated using the probability of exceedance methodology used by the Corps.

Costs and benefits were then analyzed over a 50-year period. The net present value of these costs and benefits were compared to evaluate project cost-effectiveness. Benefit-cost ratios of 1 or greater are considered cost-effective. Table ES-2 shows the National Economic Development benefit-cost analysis results. The costs in Table ES-2 include construction, financing operation and maintenance costs on a net present value (NPV) basis.

Table ES-2			
Benefit-Cost Ratios, 50-Year Period, 2010 Dollars			
	Benefit	Cost	Benefit-Cost Ratio
Flood Reduction Project			
Upper Chehalis	\$235,318,195	\$206,766,205	1.14
South Fork	\$70,425,166	\$105,352,985	0.67
Both Projects	\$274,267,210	\$312,119,190	0.88
Multi-Purpose Project			
Upper Chehalis	\$334,439,952	\$296,479,010	1.13
South Fork	\$90,058,967	\$162,338,251	0.55
Both Projects	\$387,408,239	\$458,817,261	0.84

The studies conducted to date are high level reconnaissance studies. Because hydraulic modeling does not yet exist for Thurston and Grays Harbor County, these results provide the benefits for Lewis County only. If the Flood Authority decided to proceed with any of the project options, more detailed engineering and geotechnical work and economic analysis would be required to refine these results and update the benefit/cost ratio. Additional hydraulic modeling in both Thurston and Grays Harbor County would also be helpful. In addition, the environmental studies contracted by the Flood Authority have not yet been finalized. These studies will provide important information that will need to be incorporated into the analysis. Project permitting, environmental assessment under SEPA/NEPA, the Endangered Species Act, and other relevant statutes and regulations would also be required in future development phases.

⁷ U.S. Army Corps of Engineers, June 2003. *Centralia Flood Damage Reduction Project Chehalis River, Washington. Final General Reevaluation Report.* Appendix D: Economics. Department of the Army. Washington D.C.

Conclusions and Recommendations

The benefit-cost analysis completed using the Corps’ Methodology indicated that the Upper Chehalis project may be cost-effective as a flood reduction project or multi-purpose project.⁸ The South Fork Chehalis benefit-cost ratios are not favorable.

The Phase IIB economic analysis included several benefits and costs analyzed according to Corps’ methodology. The scope of this study included analysis of costs and benefits of the projects following two additional benefit-cost constructs: the Alternate Analysis, and Regional Analysis. These additional benefits might be attributable to the projects, but information regarding these benefits needs further refinement beyond the scope of this study.

Table ES-3 summarizes the benefits and costs in NPV over a 50-year planning period.

Table ES-3 Benefit-Cost Ratios, 50-Year Period 2010 Dollars			
	Benefit	Cost	Benefit-Cost Ratio
Upper Chehalis			
Flood Reduction (NED)	\$235,318,195	\$206,766,205	1.14
Multi-Purpose (NED)	\$334,439,952	\$296,479,010	1.13
Alternative	\$361,795,889	\$296,479,010	1.22
Regional	\$372,188,297	\$296,479,010	1.26
South Fork			
Flood Reduction (NED)	\$70,425,166	\$105,352,985	0.67
Multi-Purpose (NED)	\$90,058,967	\$162,338,251	0.55
Alternative	\$98,922,722	\$162,338,251	0.61
Regional	\$101,459,404	\$162,338,251	0.62
Both Projects			
Flood Reduction (NED)	\$274,267,210	\$312,119,190	0.88
Multi-Purpose (NED)	\$387,408,239	\$458,817,261	0.84
Alternative	\$423,627,932	\$458,817,261	0.92
Regional	\$437,225,878	\$458,817,261	0.95

Note: only the Multi-Purpose structures are considered under the “Alternative” and “Regional” cases because the multi-purpose structures provide potential additional benefits that can be examined.

⁸ This conclusion is based on the analysis using the Corps’ methodology, the best available data accessible to the authors, and the assumptions made, with allowances made for contingencies.

Introduction and Scope

In 2009, the Chehalis River Basin Flood Authority (Flood Authority) contracted EES Consulting, Inc. (EESC) to evaluate the feasibility of flood retention structures in the Chehalis River Basin. The feasibility studies have been conducted in phases, each building on the information developed and conclusions from the previous phases.

The purpose and scope of work for Phase I of the feasibility studies was to assess the possible benefits of developing water retention facilities, or flood reduction structures, in Lewis County, primarily the Chehalis River Basin. In particular, two sites were chosen to be reviewed at a level of detail consistent with a conceptual level study. One site is located upstream of Pe Ell on the Upper Chehalis River, the other is on the South Fork of the Chehalis River.⁹ Total estimated flood storage for both sites is approximately 100,000 ac-ft. The Phase I study analysis showed that retention structures at these sites could provide a cost-effective means to reduce the frequency, severity, and associated impacts of flooding in the Chehalis River Basin.

The scope of work for Phase IIA of the feasibility studies included a geotechnical study, which concluded that no geotechnical impediments exist to the construction of water retention facilities at either site. In response to community interest, the Flood Authority contracted EESC to refine the economic and engineering estimates developed during Phase I.

The scope of work for these Phase IIB economic and engineering analyses is described in detail below.

Scope of Phase IIB

Phase IIB includes two sets of analyses: further development of the engineering concepts, and an updated and refined economic analysis based on feedback received in Phase I.

The Phase IIB Feasibility Study is one component of the second phase of preliminary studies initiated by the Flood Authority to explore options for constructing flood reduction structures on the Chehalis River. After an initial study examined several sites, continued study has focused on two locations: one site located upstream of Pe Ell on the Upper Chehalis River, and a second site on the South Fork of the Chehalis River. The purpose of this Phase IIB Feasibility Study is to further examine the cost-effectiveness of the proposed projects. The results of the Phase IIB study may be used to determine if other studies should be conducted and if a more detailed study of the benefits and costs of the projects is warranted.

⁹ Phase I of the analysis considered flood storage structure locations on the Newaukum River as well as the Upper Chehalis and South Fork Chehalis. The Newaukum River locations did not provide significant storage opportunities. The proposed sites at the Upper Chehalis and South Fork Chehalis were selected based on favorable topography as well as maximum drainage area.

When the Phase IIB study began, the scope included preliminary design and cost estimates for two proposed multi-purpose structures. The multi-purpose structures would provide flood water retention, storage for summer flow augmentation, and hydropower. After consultation with the U.S. Army Corps of Engineers, the Flood Authority asked EESC to also develop cost estimates for structures designed for flood retention only (single-purpose structures). This Phase IIB report includes a refined analysis of potential configurations of structures at the two sites using the geology and geotechnical results of the Phase IIA studies. The Phase IIB Preliminary Feasibility study¹⁰ required conceptual drawings of the two proposed structures showing preferred location, cross-sections, and locations of outlet works and spillways. Once the conceptual drawings were developed, the scope required a refined estimate for construction costs for both single and multi-purpose structures. The multi-purpose structures were designed for flood water retention, to provide water storage for summer flow augmentation, and to allow the future addition of hydropower generation. These engineering cost estimates were then to be used as input to the benefit-cost analysis.

The purpose of the Phase IIB economic analyses was to update the benefit-cost analysis according to methodology consistent with studies conducted by the Corps. The Phase IIB scope, therefore, included a benefit-cost analysis using the Corps' *Principles & Guidelines* methodology.¹¹ In addition to the *Principles & Guidelines* methodology, two other analyses were used to examine additional costs and benefits that are important to local, state, and regional interests; particularly the environmental value of the proposed facilities.¹²

Fisheries impacts are being evaluated in a Fish Study being conducted by Anchor QEA under separate contract; the results of this study are expected in June 2011.

Engineering Concept Development

The first part of Phase IIB included an update to the engineering and cost estimates of the proposed structures. During this work, the Flood Authority asked for analysis of both single purpose flood retention structures only, and multi-purpose structures as well.

In Phase IIA of the feasibility studies, EESC and its subcontractor, Shannon & Wilson, characterized the site geology and geotechnical information at the foundations and abutments for the potential retention structures, developed soil and rock data to help guide conceptual design, and identified any potential geotechnical “fatal flaws” associated with the two proposed retention sites. The results of this work are presented in two reports written by Shannon & Wilson geotechnical consultants: a Geology Report¹³ and a Geotechnical Report.¹⁴ These studies

¹⁰ Part of the scope of the overall study; results included at Appendix B of the Phase IIB Feasibility Study.

¹¹ See Notes 3 and 4, *supra*.

¹² Source materials used in preparing this report are available upon request.

¹³ Shannon & Wilson. *Geologic Reconnaissance Study Proposed Chehalis River and South Fork Dam Sites*. Seattle, WA. October 27, 2009.

¹⁴ Shannon & Wilson. *Reconnaissance-Level Geotechnical Report Proposed Chehalis River and South Fork Dam Sites*. Seattle, Washington. October 28, 2009.

identified several issues at each potential site, but concluded that these issues could be effectively addressed during design and through engineering solutions. No fatal flaws, such as unsuitable foundations, or active earthquake faults under the potential sites, were identified. However, these conclusions will eventually need to be confirmed through sub-surface investigations, such as core drilling, and addressed with engineering solutions in a future scope of work if the Flood Authority or a successor entity decides to move ahead. This kind of field work was not included in the Phase IIB scope of work.

Phase IIB includes a refined analysis of potential configurations of the two structures utilizing the geology and geotechnical results of the Phase IIA studies. The Phase IIB Preliminary Feasibility study report presents the conceptual drawings of the two proposed structures showing preferred location, cross-sections, and locations of outlet works and spillways. These conceptual drawings were then used to refine construction cost estimates for both single and multi-purpose structures. The single purpose structures are for the retention of flood waters only. The multi-purpose structures were designed for flood water retention, to provide water storage for summer flow augmentation, and to allow the future addition of hydropower generation if desired. These engineering cost estimates were then used as input to the benefit-cost analysis.

Economic Analysis

The second part of Phase IIB includes updating the economic analysis in a manner that complies with Corps standards and uses new or additional information from the Corps and other stakeholders. The economic analysis performed in Phase I of the feasibility studies used available data to determine if the Chehalis River Water Retention project had economic viability. No additional work on the economic analysis was performed during Phase IIA. Phase IIB focused on updating the economic analysis with new or additional information and input from the U.S. Army Corps of Engineers and other stakeholders.

After the Phase I benefit-cost study was circulated, several areas were identified for additional refinement. Consultation with the Corps during Phase I resulted in a plan for updating the benefit-cost analysis to be consistent with the methodology required by the Water Resources Development Act (WRDA) for federally funded projects. Specifically, the updated benefit-cost analysis conforms to the *Principles & Guidelines* methodology.

Restricted Scope of Study

Note that the analysis for this Phase IIB Feasibility Study was performed using methodologies and following the *Principles & Guidelines* methodology for national economic benefit-cost analyses and only considers a strictly defined set of parameters; not all conceivable topics were addressed nor all possible analyses performed. As befits its preliminary and limited nature, this feasibility study focused on direct economic costs and impacts at a relatively coarse level. In many cases, the analysis relied on assumptions and analogous situations on a “best available information” basis as opposed to targeted, independent study. Independent studies will be conducted as needed if the project moves forward.

Specifically, the Corps' methodology does not include the monetization of environmental costs or benefits for inclusion in a cost-benefit analysis.¹⁵ Fisheries and related environmental impacts are being evaluated in a Fish Study being conducted by Anchor QEA under separate contract, which will consider the biological and ecological issues in more depth. It is to be expected that results from this focused Fish Study may produce outcomes concerning the economic costs of fisheries impacts and/or mitigation that differ from the estimates and assumptions used in this Phase IIB Feasibility Study; such results will be considered at the appropriate points in subsequent planning and decision-making. These issues will also be thoroughly addressed in the environmental review and permitting processes under NEPA and SEPA.

The purpose of this Phase IIB Feasibility Study is not to determine whether the project, or some variant thereof, should be built. It is meant instead to make a threshold determination whether further study is warranted and to help focus where such study is most likely to produce informative results.

¹⁵ The Corps' methodology also does not consider such factors as cultural (tribal) hunting and fishing at this stage of analysis, treating them instead as "Other Social Effects" subject to evaluation and discussion at other points in the planning, permitting, and environmental review processes.

Report Organization

The remainder of the Phase IIB report is broken into four major pieces.

First, the report summarizes the site hydrology from the hydrological data obtained from the USGS.

Second, the report then provides updated cost information. This includes a summary of updated engineering concepts and designs, followed by project cost information. The full engineering report is included as an appendix to this report (Appendix B).

Third, the report incorporates the updated cost estimates into the economic analysis to update the benefit-cost ratios. The economic analysis is described in the following sections:

- Benefit-Cost Methodology: Provides background on the economic analysis methodology used by the Corps and an alternative methodology used in select parts of the United States.
- Benefits Estimated Using Corps Methodology: Follows the Corps methodology to monetize benefits of flood reduction structures.
- National Economic Benefit-Cost Analysis: Focuses on only national benefits and costs as prescribed by the *Principles & Guidelines* to complete an economic analysis.
- Alternative Analysis: Uses precedents from other parts of the country to monetize environmental effects from the projects.
- Regional Analysis: Incorporates regional benefits to the benefit-cost analyses in the previous sections.
- Benefit-Cost Analysis Summary: Compares all benefit-cost ratios in the report and varies cost assumptions to test the robustness of results.

Finally, the report summarizes the overall findings and recommends further review and refinement.

Hydrology Data and Historic Flows

This section describes the hydrology of the two sites and historic flows under flooding conditions based on available data. Northwest Hydraulic Consultants (“NHC”) used the full period of record, as presented below, to determine flood return intervals. The periods of record used in the computation of flood flow quantiles were different for different gages but all were extended to include the December 2007 flood event. For the updated work completed in 2010, NHC used flow data up through January 2009, including the December 2007 event.

Upper Chehalis Site

Flow and Gage Height Characteristics

This report relies on data describing flows recorded at the U.S. Geological Survey (USGS) gage near Doty. The Doty gage is used for the Upper Chehalis site because it is the gage located closest to the site of the proposed structure. The use of this gage is different from the Phase I analysis, which relied on the gage near Grand Mound because the Doty gage was washed out in the 2007 flood. However, feedback received on the Phase I report indicated that the Doty gage would better describe the hydrology at the two sites.¹⁶ The analysis for the Phase IIB report therefore uses the Doty gage data, including the USGS estimate for the 2007 flood. Subsequent analyses as the proposed project moves toward the design stage may incorporate data from additional gage stations, including new information obtained from gages to be installed by West Consulting as part of the Early Warning System.

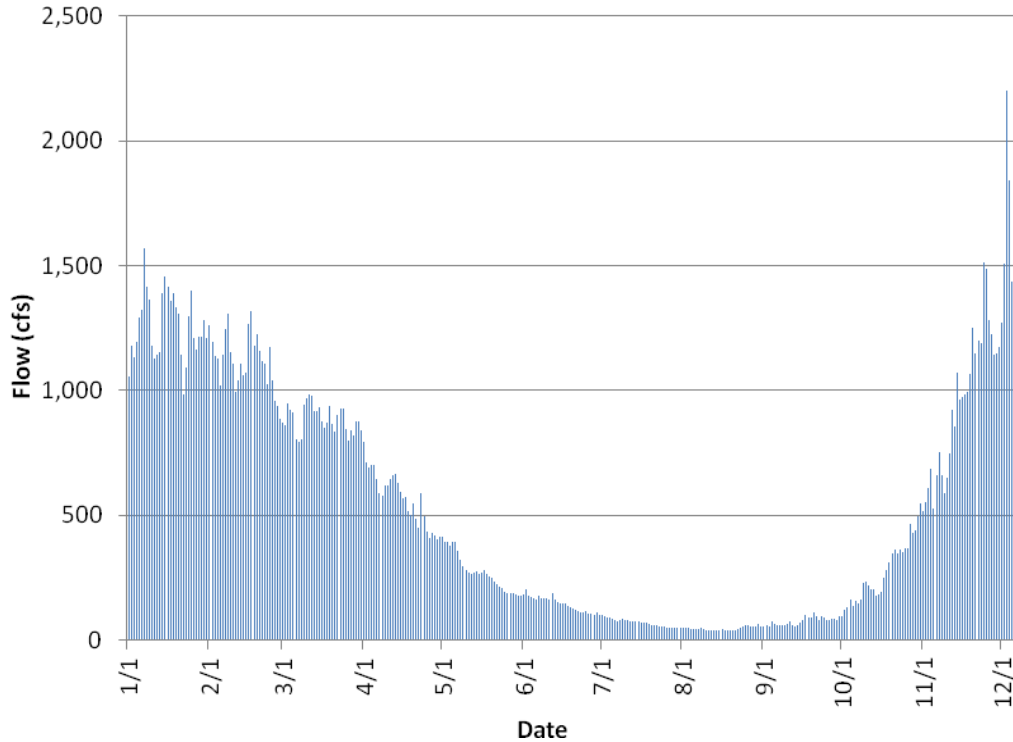
This section first describes historic average daily flow, followed by a description of historic crests and associated maximum flows.¹⁷ It should be noted that the drainage area for the gage near Doty is 113 square miles, whereas the drainage area at the proposed retention site is estimated at 68.8 square miles. Average daily flows from the Doty gage are illustrated in Figure 1. These long-term average daily flows are for the period of record from January 1940 to June 2008. Average daily flows are as low as 23 cfs in August and as high as 2,201 cfs in December.¹⁸

¹⁶ Beecher, Hal. Comments from the Washington Departments of Ecology and Fish and Wildlife on Chehalis River Retention Structures Scoping Document and Proposed Studies. Enclosed in the Department of Ecology letter to the Flood Authority Subcommittee. January 7, 2009.

¹⁷All flow data is from U.S. Geological Survey (USGS) information.

¹⁸ Maximum average daily flow likely influenced by December 2007 flood event. According to the USGS, estimated peak flows for this event are approximately 63,100 cfs.

**Figure 1
Upper Chehalis near Doty - Long Term Daily Average Flows**



The National Oceanic and Atmospheric Administration (NOAA) makes river gage data available on the National Weather Service website. According to the Probabilistic Flow Forecast for the Doty chart, flood discharge begins at a rate of 12,988 cubic feet/second (cfs). This is a maximum weekly rate. NOAA defines the flood stage at 13 feet on the Doty gage. Table 1 illustrates the historic crests for the gage near Doty. The peak flow column provides the corresponding data from the USGS.

**Table 1
Chehalis River Near Doty Historic Crests**

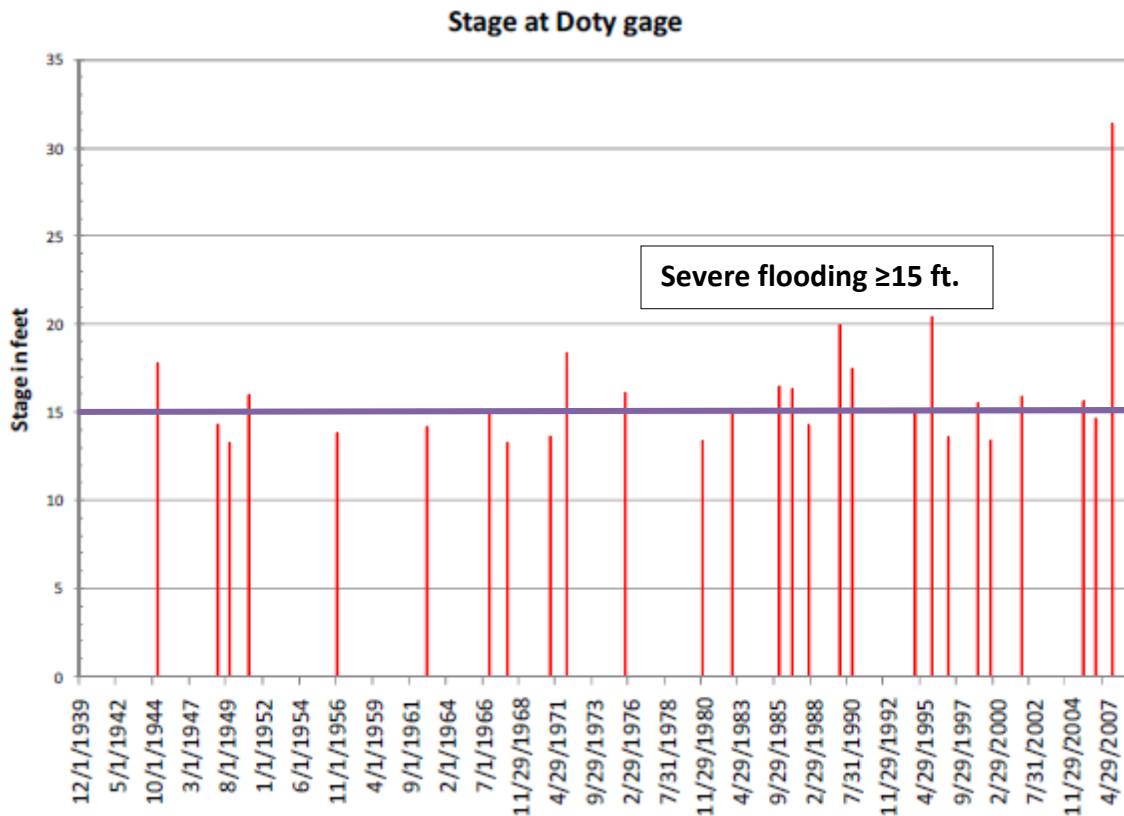
Gage Height, Feet⁽¹⁾	Peak Flow, cfs	Date
31.31	63,100 ⁽²⁾	12/03/2007
20.37	28,900	02/08/1996
19.96	27,500	01/09/1990
18.36	22,800	01/20/1972
17.80	21,400	02/07/1945
17.45	20,600	11/24/1990
16.38	Not available	01/08/2009
15.90	16,600	12/16/2001
15.58	16,000	01/30/2006
15.54	16,300	02/24/1999

(1) 12020000 Chehalis River Near Doty, WA, USGS

(2) Estimated by USGS based on high water mark because gage washed out in 2007 flood

Figure 2 illustrates the gage height data from Table 1 over the period of record. Figure 2 shows that severe flooding, defined here as gage heights ≥ 15 feet where overbank flooding first occurs at 13 foot gage height, has become more frequent in the last couple of decades. This figure shows that while the frequency of occurrence of overbank flooding has not substantially changed, the severity of flood events has increased in recent years.

Figure 2
Chehalis River near Doty
Historic Gage Height ≥ 13 Feet¹⁹



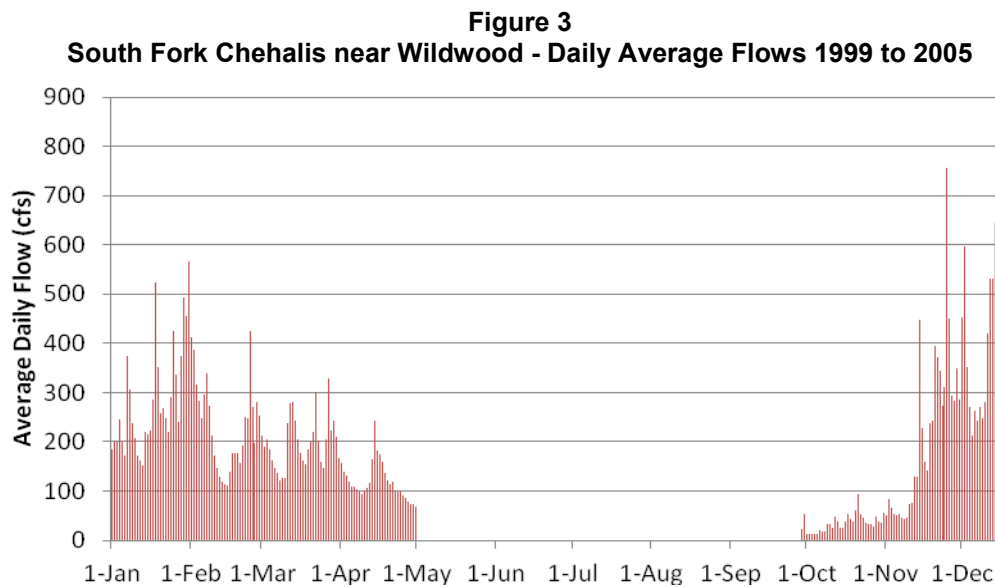
South Fork Chehalis Site

Flow and Gage Height Characteristics

For the South Fork site, the Wildwood gage is the closest gage. The gage near Wildwood records daily flow information for October through April only. The drainage area at this gage is 27 square miles, while the drainage area at the proposed South Fork site is estimated at 22 square miles.

¹⁹ Olson, Patricia. *Comments from WSDOT, Ecology, and Fish and Wildlife, Chehalis River Flood Water Retention Project Phase IIB Feasibility Study* (Nov. 10, 2010).

Average daily flows from the South Fork Chehalis gage near Wildwood are shown in Figure 3. These average daily flows are from a short period of record beginning in 1999. Average daily flows are as high as 800 cfs in December.



Historic peak flow on the Wildwood gage occurred on December 3, 2007 with a maximum discharge of 12,200 cfs.²⁰ Prior to 2007, the highest discharge occurred on February 8, 1996 at a rate of 5,620 cfs.

The Wildwood gage data are shown here to illustrate the variability of flows during the months that data were collected. However, due to the short period of record for this gage and the data gaps from May through September, EESC relied on Doty gage data adjusted for drainage area to analyze hydrology at the South Fork site.

²⁰ U.S. Department of the Interior, U.S. Geological Survey. *USGS Real-Time Water Data for Washington* Available online: <http://waterdata.usgs.gov/wa/nwis/uv/> Page Contact Information: Washington Water Data Support Team . Page Last Modified: 2011-03-14 13:39:53 EDT.

Summary of Engineering Concepts and Costs

This section describes the general locations, characteristics, and operations of the flood reduction structures. Two different options for each site are described in this section: a flood-reduction only design and a multi-purpose design. The Phase IIB scope originally tasked EESC engineers with the design and cost estimates for multi-purpose structures. However, EESC engineers were asked to also develop design and cost estimates for flood-reduction only structures. The flood-reduction only structures are discussed first, followed by discussion of the multi-purpose structures.

Locations

EESC engineers used digital mapping with 2 foot contour intervals provided by Lewis County to identify structure locations and alignment at both sites. The sites were chosen based on the topography to maximize the drainage area while allowing for sufficient abutment height for water storage.

Due to a more detailed analysis regarding site topography, the Upper Chehalis site was revised slightly from the original location studied during the Phase I work. In Phase IIB, the structure was moved approximately 1,500 ft downstream and the axis rotated. This adjustment resulted in a slightly shorter crest length and a much more desirable alignment for a tunnel through the hills located on the left side of the structure (from downstream side). The tunnel will allow for water diversion past the structure.

The South Fork Structure is in approximately the same location as identified in Phase I but was rotated slightly in Phase IIB due to improved topographic data.

Figures 4 and 5 illustrate the general locations of the two sites.

Figure 4
Project Map Location

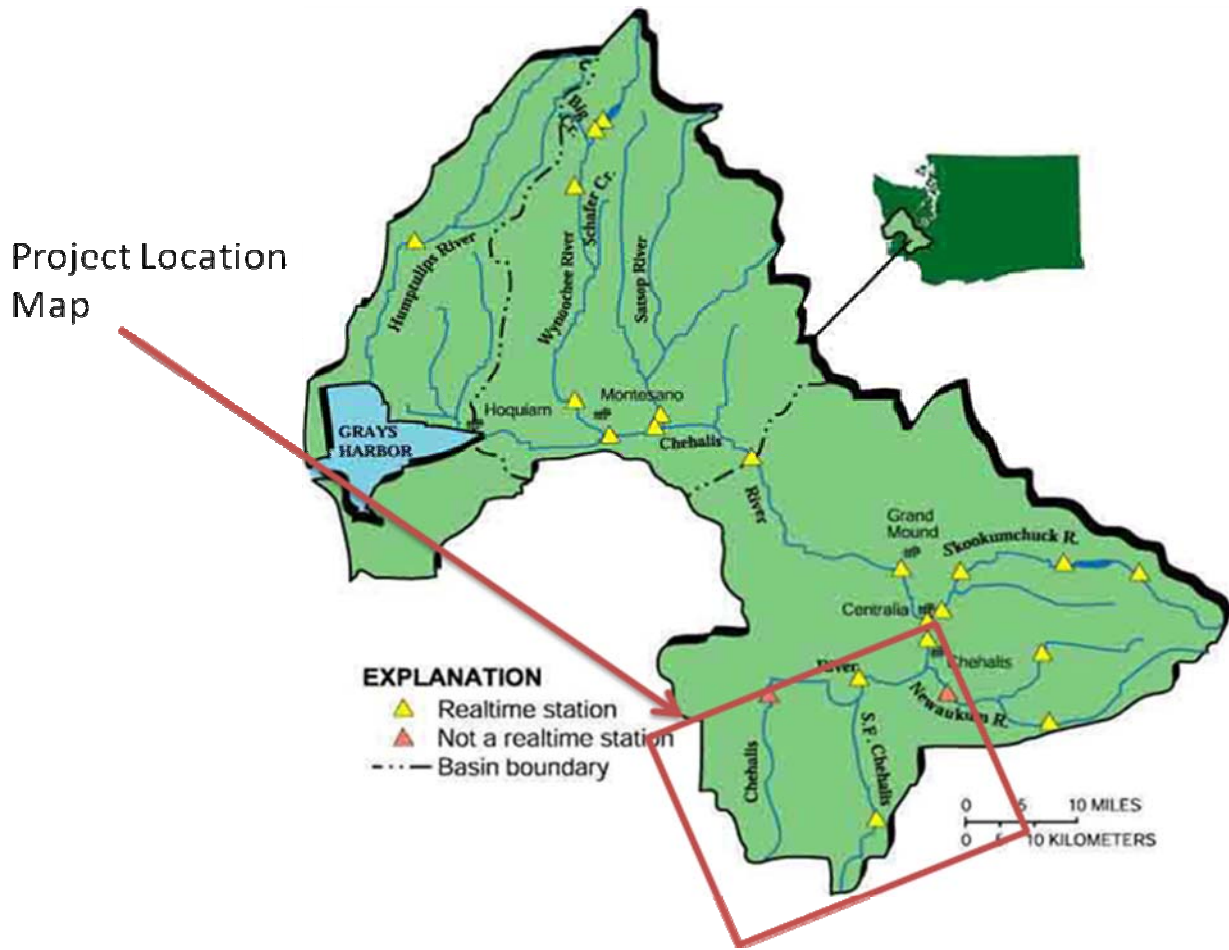
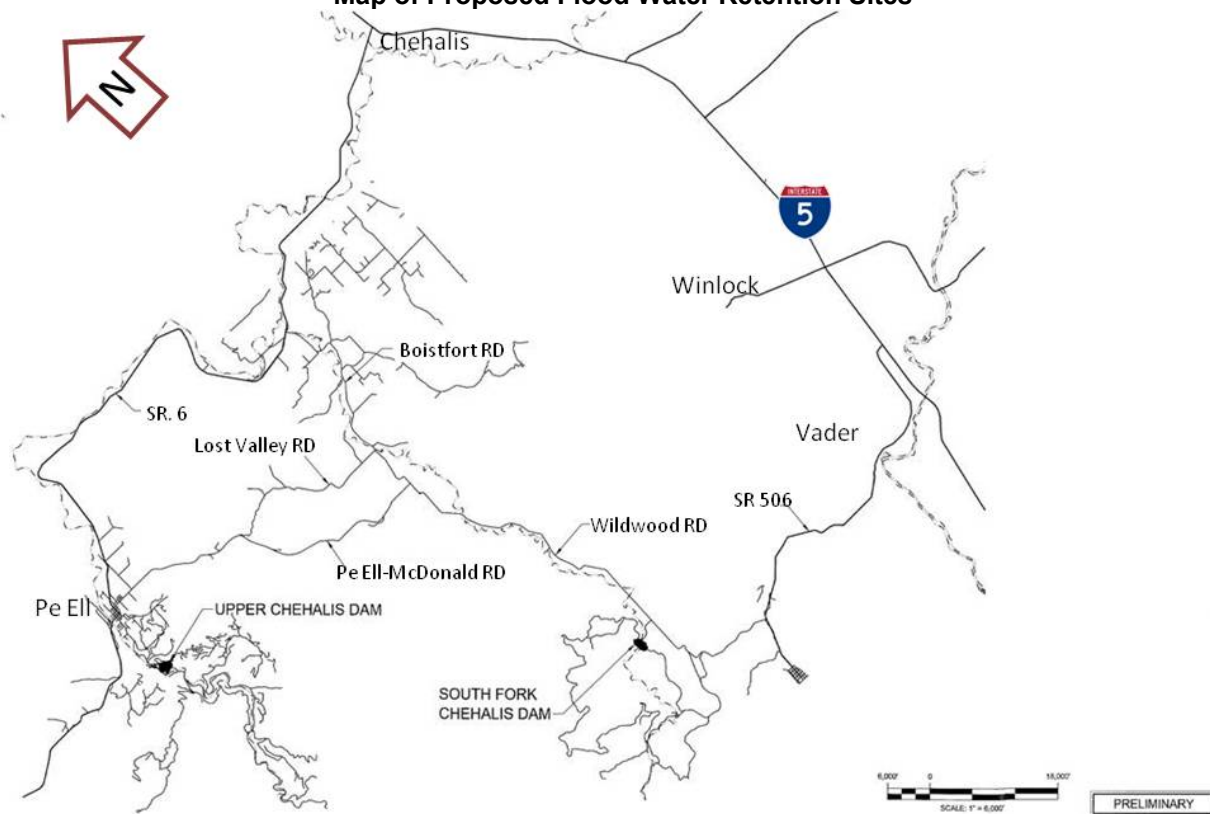


Figure 5
Map of Proposed Flood Water Retention Sites



Flood Reduction Structure Characteristics

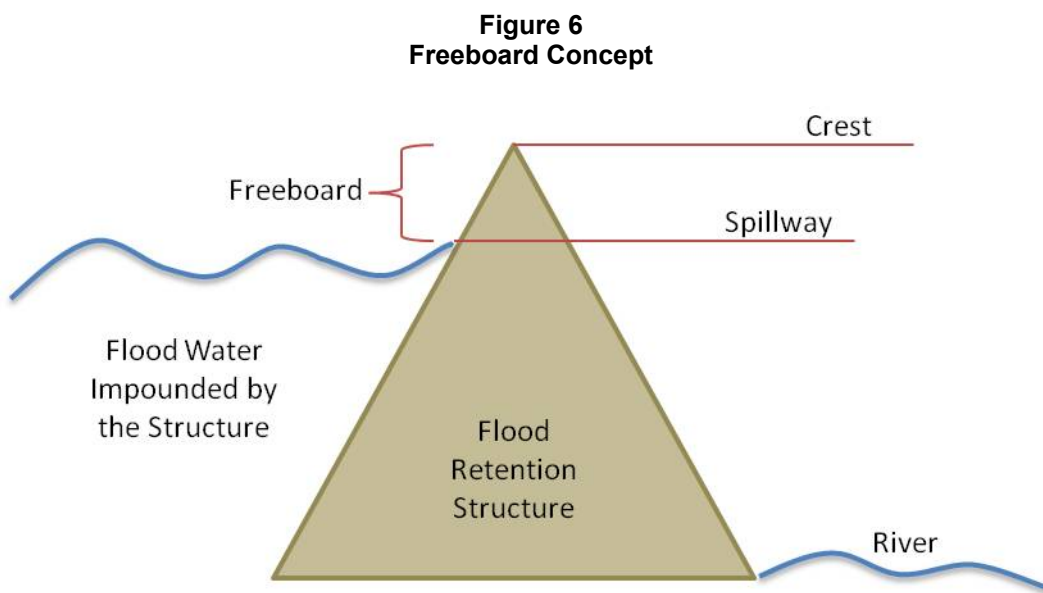
The physical characteristics of the site topography are suitable for an earthfill structure in each location. The topography is characterized by low rolling hills and both structures are located in narrow valleys close to where those valleys open to broader flood plains. The steep valley walls have resulted in landslides at or near each of the proposed sites; therefore, erosion and related issues would need to be addressed in more detail as the project planning progresses.²¹

Shannon & Wilson suggest four types of structures are feasible, three of which are earth or rock filled. Although they analyzed concrete structures, their work concluded that such structures would not be practical or economical given the site conditions. The proposed flood reduction structures are run-of-river structures where natural flows are released year round, except during flood events. Each structure would have a spillway and fish passage structures. Spillways are located near the tops of the structures so that water can be safely released in the event that flood waters cannot be contained. The spillways ensure that the structures would not be overtopped.

²¹ Shannon & Wilson, October 27, 2009. The structures were then modified based on correspondence and recommendation given additional information about the sites; see Appendix B and associated correspondence from Shannon & Wilson.

Storage Volume and Structure Height

Based on topographical data, a storage volume curve was produced for each structure. The storage volume curve shows the relationship between water surface elevation (height) and storage volume. This storage volume curve is used to determine a structure height that satisfies the flood storage criteria. The spillway is located at the elevation where total storage volume is equal to flood reduction storage. Next, the structure height is increased so that the crest is higher than the elevation of the spillway to ensure that water does not overtop the structure. This additional height is known as freeboard. Freeboard is the difference in height between the spillway level and the structure crest. Figure 6 illustrates the freeboard concept by showing a cross section of an example structure.



Upper Chehalis Structure for Water Retention Only

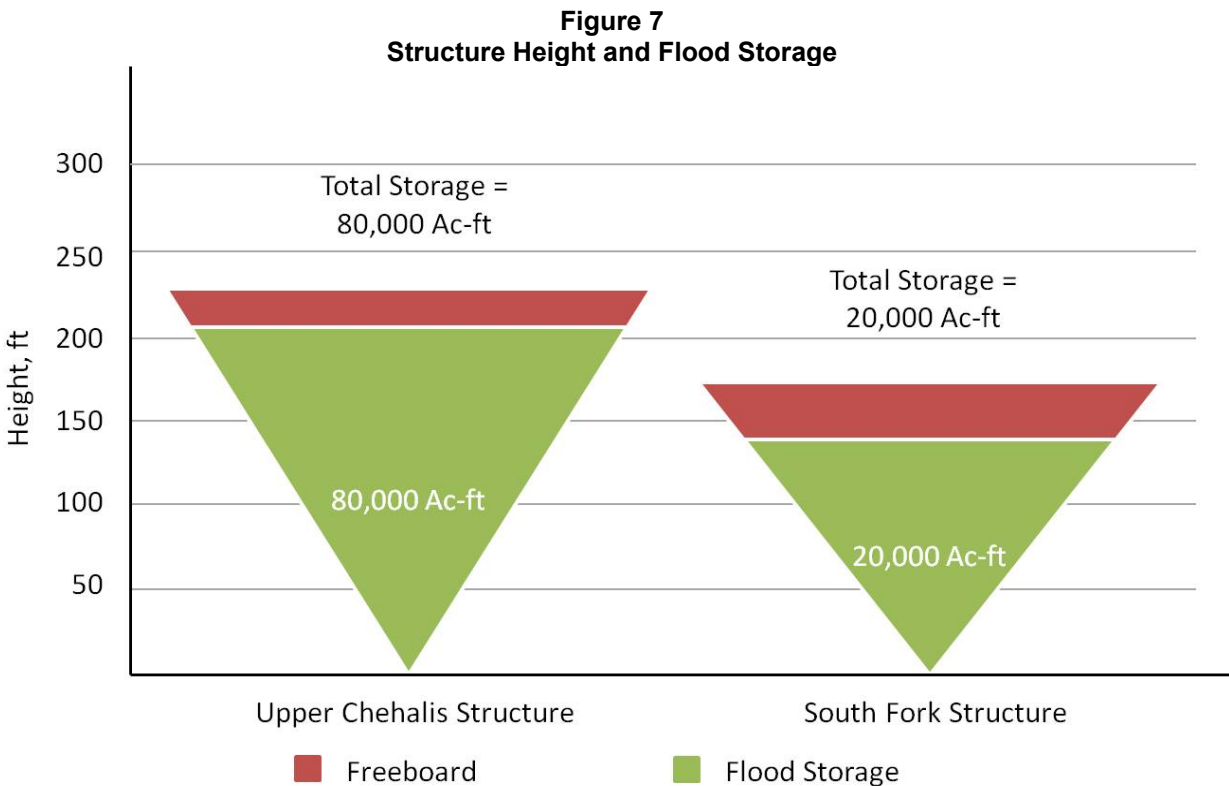
The Upper Chehalis Flood Water Retention Structure assumes 80,000 ac-ft of storage available for flood water retention. The base of the structure is at 432 feet (elevation). When full of flood waters, the reservoir elevation is 650 feet. The structure crest would be at elevation 670, which allows for an additional 20 feet (for freeboard) above the height allocated for flood water retention.²² The maximum height of the Upper Chehalis Flood Reduction Structure is estimated to be 238 feet.

²² This freeboard, and the freeboard for the South Fork Chehalis Structure, as discussed in the next section, are calculated to prevent overtopping during extreme flooding events, i.e., to provide 0.5 feet of freeboard at the maximum surface elevations calculated for a each site during such events. This would be 669.5 feet at the Upper

South Fork Chehalis Structure for Water Retention Only

The South Fork Flood Water Retention Structure assumes 20,000 ac-ft of storage available for flood water storage. The base of the structure is at 420 ft (elevation). When full of flood waters, the reservoir elevation is 560 ft. The structure crest would be an additional 30 feet above the height allocated for flood reduction (30 feet for freeboard) at 590. The maximum structural height of the Upper Chehalis Flood Reduction Structure is estimated to be 170 ft.

Figure 7 illustrates the height of the structures according to storage volume requirements.



Flood Reduction Project Operations

Flooding in the Chehalis River Basin is caused mainly by rainstorms during winter months. It is anticipated that facility operators would hold back water at the structures as weather forecasters detect storm events that may cause flooding. Once a flood event has ended, the reservoir levels are gradually lowered as operations return to normal and natural flows are released.

Chehalis site and 589.5 feet at the South Fork Chehalis site. The amount of freeboard, crest elevations, and spillway capacities will need to be reviewed during design development.

Multi-Purpose Structure Characteristics

The physical characteristics of the site topography are also suitable for an earthfill structure in each location with multi-purpose capabilities. The principal project features for the flood-reduction-only projects are the structure, spillway, and tunnel. Similarly, the principal project features for the multi-purpose projects are the structure, spillway, intake tower and tunnel. Each structure would have a spillway, outlet works, and fish passage. Spillways are located near the top of each structure so that water can be safely released in the event that flood waters cannot be contained. The spillways ensure that the structures would not be overtopped. Outlet works are located at the outlet side of the tunnel and allow for the regulated release of water. Intake towers will be located in the reservoirs so that water that is released through the tunnel, and eventually through the outlet works, can be drawn from varying depths (to regulate temperatures, levels of dissolved oxygen, etc.).

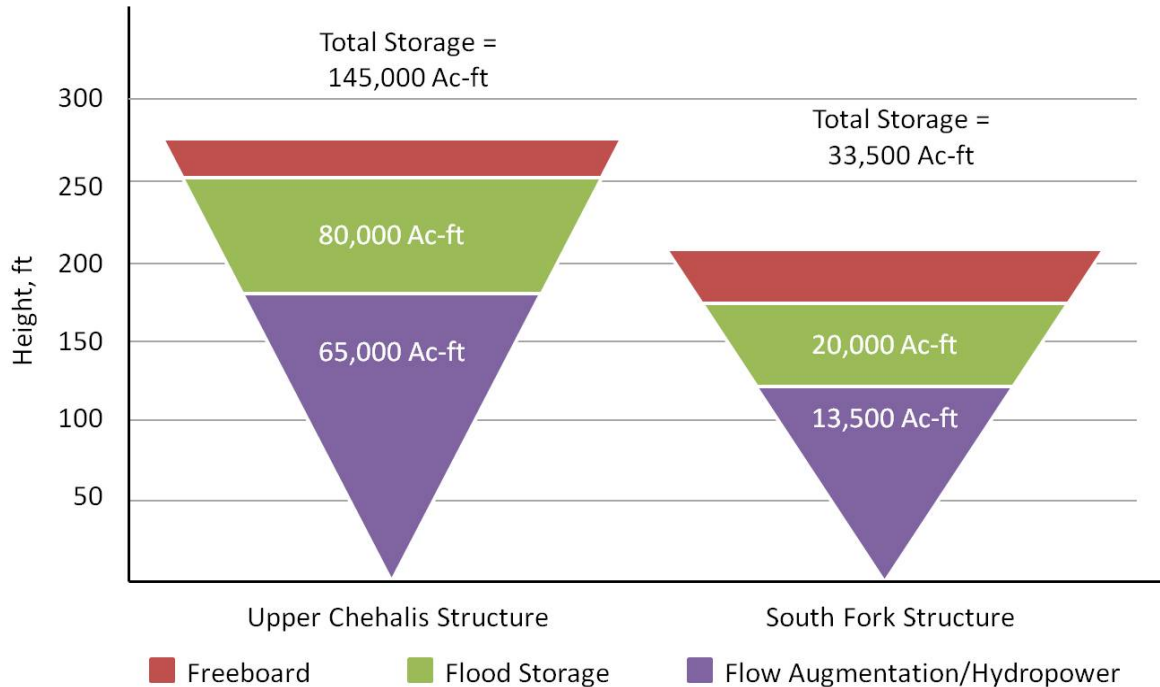
Storage Volume and Surface Area

Based on topographical data, a storage volume curve and a reservoir surface area curve were produced for each structure. The storage volume curve shows the relationship between water surface elevation (height) and storage volume. The surface area curve shows the relationship between water surface elevation and reservoir surface area. The storage volume curve is used to determine a structure height that ensures sufficient storage capacity to contain flood waters and additional room for stream augmentation and hydropower.

Multi-Purpose Structure Height

The height of each structure was determined by first selecting the water surface elevation needed for flood management, followed by the additional height needed for flow augmentation and hydroelectric energy production. The spillway was located at the elevation where total storage volume is equal to flood water storage and flow augmentation/hydropower storage. Finally, the structure height was increased to add freeboard to ensure water does not overtop the structure. Figure 8 illustrates the structure height for each of the proposed structures. Storage volumes are also shown for flood storage and flow augmentation/hydropower.

**Figure 8
Multi-Purpose Structure Height**



The maximum operating water surface elevations for hydro operation was selected based on two criteria: enough storage to augment summer flows and a reasonable operating level for production of hydropower. An energy production model was developed for each project to maximize hydropower output with respect to the amount of water available on an average daily flow basis.

Upper Chehalis Site with a Multi-Purpose Structure

The characteristics of a multi-purpose structure depend on the volume needed for both hydropower generation and flood control.

The maximum water surface elevation selected for the Upper Chehalis site to generate hydropower is 195 feet (ft) from the base of the structure. The maximum volume of water for the hydro operation would be approximately 65,000 acre-feet.

Capturing an additional 80,000 ac-ft for flood reduction would require raising the water surface by 65 ft (assuming the reservoir is full at 195 feet). An additional 65 feet in structure height corresponds to the maximum reservoir capacity of 145,000 ac-ft (total). When completely full with flood waters, the Upper Chehalis reservoir would have a surface area of approximately 1,450 acres.

The structure crest would be an additional 20 feet above the height allocated for flood reduction (20 feet of freeboard). See Figure 8 above. The maximum height of the Upper Chehalis structure is estimated to be 288 feet.

South Fork Chehalis Multi-Purpose Structure

Similarly, the characteristics for the South Fork Chehalis multi-purpose structure also depend on the amount of water needed for flood reduction and hydropower.

The maximum water surface elevation selected for the South Fork site for hydro generation is 120 ft from the base of the structure. At this height, the storage volume would be approximately 13,500 ac-ft. An additional 20,000 ac-ft of storage is required for the flood reduction purpose of this structure; therefore, the maximum reservoir capacity is 33,500 ac-ft. For 33,500 ac-ft of storage, the spillway crest would be 170 ft above the base of the structure. The structure crest would be 200 feet high allowing 30 ft for freeboard (see Figure 8 above). The South Fork structure would have a maximum height of approximately 200 ft.

Flood Reduction Project Operations

Similar to the flood reduction only projects, structure operators will begin to release water from the reservoirs as soon as weather forecasters detect storm events that may cause flooding. The additional releases in anticipation of a flood event will keep the reservoirs from filling quickly and releasing water through the uncontrolled spillway. The maximum amount of water that can be safely released has not been calculated for this study and will need to be modeled in later phases. It was assumed at 1,000 cfs and 350 cfs could safely be released for the Upper Chehalis and South Fork Chehalis structures respectively. Please see Appendix B for additional information on release assumptions.

Once a flood event has ended, the reservoir levels are gradually lowered as operations return to normal.

Hydroelectric Project Operations

Flow data based on the USGS gage near Doty was used to develop an energy production model. The gage flows were correlated to the sites using a ratio of the drainage areas. The drainage area of the Doty gage is 113 sq mi and has a daily average discharge of 349 cfs. The drainage areas are 68.8 square miles and 22.5 square miles for the Upper Chehalis and South Fork sites, respectively.

The energy production models used the reservoir storage curves developed from the digital mapping, average daily flow from the Doty gage, and flow releases. Flow releases are based on the size of the hydropower units. The size of the desired generating units was found through an iterative process. The average daily reservoir inflow and reservoir storage curves are known. From the known data, the size of the hydropower units is changed until beginning and ending reservoir levels are equal. In other words, the selected unit sizes are based on maintaining a water budget so the reservoirs fill to the same initial level at the beginning of each year.

Because hydropower is operated during the summer months, summer flows are augmented by the amount of flow equal to the hydropower outflow minus natural average flow. These augmented flows are discussed further in the “Alternative Analysis” section where potential benefits of augmented flows are analyzed. Note that the system will be operated according to the following priorities, in descending order: flood control; optimum instream flows once optimum levels have been determined; and hydropower generation.

Upper Chehalis Project

The following bullets summarize the hydro generation analysis for the Upper Chehalis structure. More detail on the modeling is provided in Appendix B.

- Flows at the site can support a hydropower plant with two turbines, one rated at 8.3 MW and one at 1.7 MW, for a total capacity of 10 MW.
- Annual average energy production is calculated at 39,952 MWh.
- According to current modeling, the lowest flows would be 140 cfs in an average water year. In drier years, the minimum release might be as low as 20 cfs. This compares to 23 cfs minimum flow during average years and 16 cfs during dry years based on historic data at the Doty gage. Further study is required to determine appropriate minimum instream flow releases. A placeholder of 20 cfs is used in this study until additional information is available.

South Fork Project

The following bullets summarize the hydro generation analysis for the South Fork Chehalis structure. More detail on the modeling is provided in Appendix B:

- Estimated flows at the South Fork site can support a hydropower plant with two turbines, one rated at 1.7 MW and one at 0.3 MW, for a total capacity of 2 MW.
- Annual average energy production was calculated at 7,401 MWh.
- According to current modeling, in the case of an average water year, the lowest flows would be 40 cfs. In drier years, the minimum release might be 10 cfs. Further study is required to determine appropriate minimum instream flow releases. A placeholder of 10 cfs is used in this study until additional information is available.

Summary of Structure Characteristics

Table 2 summarizes the characteristics for the flood reduction and multi-purpose structures each site.

Table 2
Comparison of Project Data

	Upper Chehalis Flood Reduction	Upper Chehalis Multi-Purpose	South Fork Flood Reduction	South Fork Multi-Purpose
Structural Height (Ft)	238	288	170	200
Hydraulic Height, Normal Operating Depth at Structure (Ft)	NA	203	NA	120
Natural Streambed Elevation	432	432	420	420
Crest Elevation	670	720	590	620
Crest Length (Ft)	1,450	1,800	1,750	1,880
Crest Width (Ft)	40	40	40	40
Base Width (Ft)	1,300	1,600	860	1,025
Volume of Construction Materials (Cubic Yards)	5,458,100	8,921,600	3,345,900	7,814,800
Total Water Storage at Elevation	80,000 acre-ft at 650 ft	145,000 acre-ft at 700 ft	20,000 acre-ft at 560 ft	35,000 acre-ft at 590 ft
Maximum Water Surface Elevation	669.5	719.5	589.5	619.5
Spillway Capacity at Elevation	50,000 cfs at 669.5 ft	50,000 cfs at 719.5 ft	24,000 cfs at 589.5 ft	24,000 cfs at 619.5 ft
Flood Storage Volume (ac-ft)	80,000	80,000	20,000	20,000

Project Costs

This section summarizes cost estimates developed by EESC engineers for structure design and construction. Operation and maintenance costs are estimated using relevant literature and verified by EESC engineers. Costs for interest during construction are also discussed.

Construction Costs

The estimated costs of development and construction of the Flood Reduction Structures are \$165,230,000 for the Upper Chehalis Project and \$93,060,000 for the South Fork Project. The estimated costs of development and construction of the Multi-Purpose Structures are \$245,060,000 for the Upper Chehalis Project and \$148,540,000 for the South Fork Project. These cost estimated include a 30% contingency factor.²³ The cost estimates developed for each project are presented in Appendix B. The total estimated costs are believed to be accurate within 30%. Shannon & Wilson reviewed the cost estimates and concurred with EESC's final assessment. In addition, feedback from the state agencies noted that "A conservative, well thought out engineering design process was followed, fully in compliance with Dam Safety's December 2009 letter of concurrence."²⁴

These estimates represent the probable project development costs as best they can be determined at this preliminary stage. The estimates are based on the Phase I investigations, preliminary drawings, material quantity take-offs, construction cost guides, recent construction bids, literature research, opinion, judgment and allowances. Note that additional reviews will need to be carried out after further site investigation takes place and development of geotechnical, seismic, hydrology, and other supporting reports for the Project have been completed. EESC requested assistance from Shannon & Wilson for unit construction costs for selected items, such as embankment and tunneling. Land and land right costs such as land acquisition, FERC licensing, state and local permits, Bonneville Power Administration coordination fees have been included, but internal agency/Flood Authority costs and legal fees have not.

The construction schedule is assumed to take about 4 years once permits have been issued. The issuance of permits will take some time in order to comply with environmental permitting requirements.

²³ Thirty percent contingency values are used in the engineering cost estimates. The amount of contingency used for this cost estimate depended on the level engineering detail and design used in developing the estimate. Not only was geotechnical information available for this project, quantities of materials for the dam and tunnel were fairly well defined. These structures account for the majority of the estimate. Based on the professional experience of EES Consulting's engineers, the level of information available, and the level of design completed under Phase IIB, a 30% contingency is considered appropriate.

²⁴ Comments from David Cummings, P.E., Department of Ecology on Phase IIB Feasibility Study (Nov. 10, 2010 Draft).

Cost Scenarios

A range of costs is included in the analysis to test the robustness of the benefit-cost analysis results. Table 3 summarizes project construction cost ranges. The base level costs are those developed by EESC engineers, including 30 percent contingency. The low costs exclude the 30 percent contingency, while the high costs include \$30 million²⁵ for environmental mitigation for each project (compared with \$18 million at each structure in the base case) in addition to 30% contingency. Higher fish mitigation costs might be consistent with the additional requirement subject to the findings of the environmental analysis currently being performed by Anchor QEA. However, at this stage of analysis such impacts, including possible impacts on the benefit-cost ratio, cannot be predicted with sufficient precision to be useful until the Anchor QEA study has been completed.

Table 3
Project Cost Estimates²⁶
2010 Dollars

	Base	Low	High
Flood Reduction			
Upper Chehalis	\$165,230,000	\$129,258,200	\$235,079,000
South Fork	\$93,060,000	\$72,800,100	\$141,258,000
Both Projects	\$258,290,000	\$202,058,300	\$376,337,000
Multi-Purpose			
Upper Chehalis	\$245,060,000	\$191,708,200	\$338,858,000
South Fork	\$148,540,000	\$116,202,600	\$213,382,000
Both Projects	\$393,600,000	\$307,910,800	\$552,240,000

For comparison, the costs estimated under Phase I of the *Chehalis River Water Retention Facilities Potential Study* (2009) were \$204,120,000 and \$131,880,000 for the Upper Chehalis and South Fork Chehalis structures respectively.

Operation and Maintenance Costs

Operation and maintenance (O&M) costs are estimated based on a 2003 report reviewing Pacific Northwest hydroelectric project O&M costs.²⁷ Production costs in \$/MWh from an 11 MW structure with storage characteristics were used for both the Upper Chehalis and South Fork

²⁵ Based on higher fish mitigation costs for possible volitional passage or other requirements.

²⁶ Fish mitigation costs are estimated based on trap-and-haul for fish passage; see Appendix B, page 25.

²⁷ EES Consulting, Inc. November 2003. *Cowlitz Falls Project Independent Review*. Completed for Lewis Country PUD. Kirkland, WA. The O&M costs were benchmarked for the multi-purpose projects at Wynoochee (\$25.44/MWh) and Cowlitz Falls (\$7.93/MWh).

Chehalis O&M costs. The structure referenced is not manned around the clock, but is remotely monitored all hours of the day. There are 4.5 full time union employees. Due to economies of scale, O&M costs for larger structures were excluded from consideration. In 2010 dollars, O&M costs for the structures were estimated at \$145,200/full time employees (FTE). Based on size and operational characteristics, the number of FTEs was estimated for each structure type. It was assumed that fewer FTE would be required to operate the Flood Reduction structures. Table 4 illustrates the results of the O&M cost analysis²⁸. When built together, it is likely that these projects would be maintained by the same crew; however, no O&M cost reductions were assumed in the case where both projects are built.

**Table 4
O&M Cost Estimates
2010 Dollars**

	FTE	Annual Cost	\$/MWh
Flood Reduction			
Upper Chehalis	5	\$798,590	NA
South Fork	2	\$290,396	NA
Both Projects	7	\$1,0881,987	NA
Multi-Purpose			
Upper Chehalis	7	\$1,016,388	\$25.44
South Fork	3	\$435,595	\$58.86
Both Projects	10	\$1,451,982	\$30.66

Interest During Construction (IDC) Costs

Interest during construction is calculated for each project assuming one quarter of the cost is financed for each of the four years of construction (25 percent in year 1, 50 percent in year 2, etc.). The net present values (NPV) of interest during construction (IDC) costs are \$9.9 and \$14.8 million for the Upper Chehalis flood water retention and multi-purpose structures respectively. Likewise, IDC costs are \$5.6 and \$8.9 million for the South Fork flood retention and multi-purpose structures.

²⁸ The study also referenced Corps report on O&M costs for hydro projects. Hall, Douglas, G. et al. June 2003. *Estimation of Economic Parameters of U.S. Hydropower Resources*. Idaho National Engineering and Environmental Laboratory, Bechtel BWXT Idaho, LLC. However, as the costs are calculated based on MW generation, the resulting O&M costs estimates were too low.

Cost Summary

Table 5 summarizes the costs for the projects.

Table 5 Project Cost Summary (Base) 2010 Dollars			
	Construction	Annual O&M	NPV IDC
Flood Reduction			
Upper Chehalis	\$165,230,000	\$798,590	\$9,917,767
South Fork	\$93,060,000	\$290,396	\$5,585,834
Both Projects	\$258,290,000	\$1,088,987	\$15,503,601
Multi-Purpose			
Upper Chehalis	\$245,060,000	\$1,016,388	\$14,709,484
South Fork	\$148,540,000	\$435,595	\$8,915,966
Both Projects	\$393,600,000	\$1,451,982	\$23,625,450

These Project costs are included in the benefit-cost analysis.

Benefit-Cost Analysis Methodology

This report presents an economic analysis and benefit-cost ratios developed using the U.S. Army Corps of Engineers' *Principles & Guidelines* methodology, which considers costs and benefits from a national perspective. Because there are numerous costs and benefits that occur on a local, state, and regional basis, this analysis also includes an Alternative Analysis. The primary addition to the Alternative Analysis is the quantified costs and benefits to the affected environment and ecosystem. A Regional Analysis of costs and benefits is also conducted from a local perspective. This section contains a brief summary of the different types of analyses, followed by greater detail for each analysis.

National Perspective

The Corps approves flood reduction projects based on the results of a benefit-cost analysis using its *Principles & Guidelines* methodology, which evaluates costs and benefits from a national perspective. Relevant national costs and benefits attributable to the proposed projects are described below. These values were monetized; non-monetized values are discussed later in this section.

Relevant National Costs

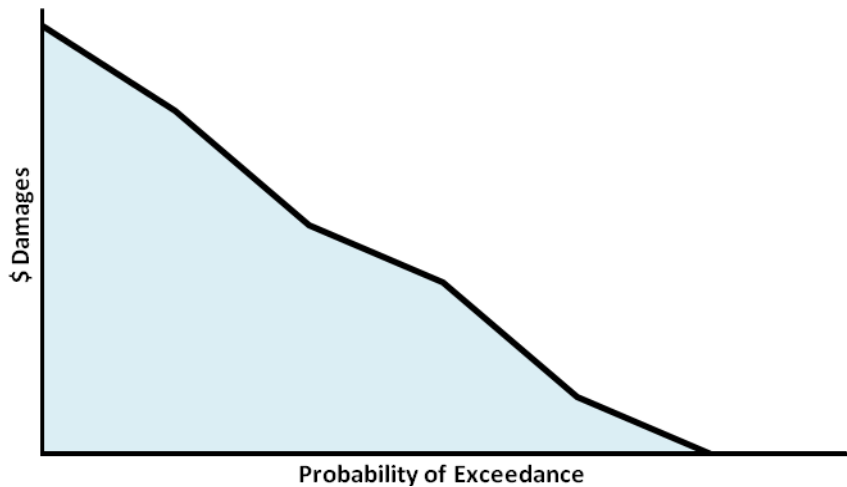
- Capital construction costs
- Operation, maintenance, and replacement
- Permitting costs, such as FERC licensing, state and local permits

National Benefits

- Reduced estimated annual damage to building structures and contents, agriculture crops and equipment
- Avoided clean-up costs
- Avoided transportation delays or detours
- Avoided infrastructure improvement or operation and maintenance
- Increased availability of water for irrigation or other use
- Value of hydropower and its renewable qualities
- Increased recreation visits

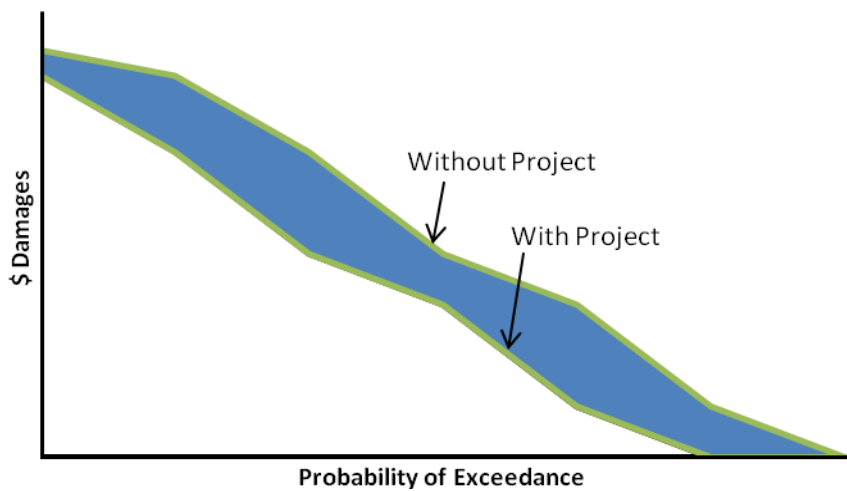
Using the *Principles & Guidelines* methodology, the Corps calculates the value of flood damages for several flood return intervals (25, 50, 100 years, etc.). The Corps methodology results in a probability of exceedance “damage curve.” Figure 9 illustrates a sample damage curve. The curve demonstrates that as the exceedance probability of a flood decreases, damages increase.

Figure 9
Example Damage Curve



Once a flood reduction project is introduced, the damage curve will shift such that damages are reduced in some or all flood events. Damage curves were estimated for two scenarios: one with the project built and one if it was not. The difference between the curves is the benefit of the project. Benefits include values such as avoided damages to building structures and contents, agriculture products and equipment, avoided clean-up costs, and avoided costs due to transportation delays and detours. Figure 10 shows a sample shift. The expected annual benefit (in dollars) of the flood reduction project is the area between the curves (blue shading) in Figure 10.

Figure 10
Example of Reduction in Damage Curve



It should be noted that the Corps' damage curves for the most recent flood events (2007 and 2009) are not available. In addition, the damage curves estimated in the 2003 Corps report only surveyed properties near the I-5 corridor in Lewis County. Over the past 20 years, however, Chehalis River flooding has resulted in flood damages from Doty to Aberdeen and not just in the I-5 corridor cities of Chehalis and Centralia. Since the Corps has not yet completed updated damage curves, EESC estimated these curves using flood depth data from Northwest Hydraulic Consultants with FEMA's GIS-based HAZUS model to determine flood damages at 10, 50, 100, and 500 year events, for the following cases:

- With Upper Chehalis Structure;
- With South Fork Chehalis Structure;
- With both structures; and
- Without either structure.

Regional Perspective

According to the *Principles & Guidelines*, however, regional benefits and costs may be included under a separate analysis when evaluating alternatives for federal funding. Regional perspectives are not valued as highly as national perspectives because regional costs and benefits often transfer from one region to another. For example, the local grocery stores in Lewis County may lose sales during a flood event; however, grocery stores in a neighboring county or state may experience increased sales. From a national benefit perspective, this transfer of sales is not counted. However, for the local grocer, the loss of sales may have a big impact and could be counted in a regional analysis.

Since the Corps is a federally funded agency, projects must have favorable economics from a national perspective. For this study, however, state and regional benefits are important, as stakeholders at the local, state and regional levels will play an important part in determining the best overall solution based on a local and regional perspective. Regional benefits and costs for this study include:

- Changes in property values and taxes
- Changes in local employment and business income
- Avoided lost business income

Qualitative Costs and Benefits

The Corps' policy as expressed in its *Principles & Guidelines* documents does not currently monetize environmental costs and benefits or other social effects (with the exception of historical properties). These effects are generally discussed qualitatively in project reports by the Corps. Although the Corps would not strongly emphasize environmental effects, these are important to the flood retention projects due to local, state, and regional interests in improving river and fisheries habitat values. Additional project-related environmental and social costs and benefits that were considered in the Alternatives Analysis and Regional Analysis are described below.

Environmental

- Changes in acreage of ecosystem coverage type, including lost terrestrial and riparian habitat, or gained lake/reservoir habitat
- Effects on fish and wildlife, including water quality changes, effects on salmon and steelhead spawning, rearing, and survival, effects of predatory animals resulting from changes in fish populations, and others
- Reduction in carbon dioxide or other air pollutants from possible eventual inclusion of hydropower²⁹

Other Social Effects

- Positive effects on historical or cultural properties, such as the reduced flooding of historic structures
- Positive urban and community impacts, such as quality of community life or population distribution
- Beneficial effects on public safety, health, or life

Alternative Analysis

The scope of work for the Phase IIB analysis calls for the quantification of environmental benefits and costs under an Alternative Benefit-Cost Analysis. The precedent for quantifying the project ecosystem benefits comes from the current work to restore the Mississippi River Delta. The Louisiana State legislature approved the use of multi-criteria methodology where the preferred alternative is selected based on all accounts (benefit-cost analysis, wetlands restoration, and public safety). As a result of the Louisiana legislature's decision, Earth Economics³⁰ is examining the dollar value of storm protection, carbon sequestration, and other ecosystem services provided by the wetlands of the Mississippi Delta. The valuation of these ecosystem and public safety benefits may show that protection and restoration of wetlands in conjunction with smaller levees is more economically efficient in providing hurricane protection than levee construction alone. Using these principles provides a construct for monetizing ecosystem benefits for the proposed projects.

Ecosystem improvements (and costs) are based on a report completed by Earth Economics for the Chehalis River Basin.³¹ The analysis valued changes in ecosystem types (terrestrial vs. lake), improved riparian habitat, and ability of the new wetlands to mitigate flood events.

²⁹ While the carbon impacts attributable to construction might offset these benefits to some unknown extent, an analysis of such effects is beyond the scope of this study and is not considered under the Corps' methodology at this stage of analysis. In any event, it is likely that the inclusion of hydropower capacity will not add substantially to the emissions associated with building the dam(s) exclusively for flood control, making it more likely that carbon benefits from hydropower will provide a net reduction in carbon emissions.

³⁰ Earth Economics is a consulting firm based in Tacoma, Washington: <http://www.eartheconomics.org/>

³¹ Batker, David, Briana Lovell and Maya Kocian. *Flood Protection and Ecosystem Services in the Chehalis River Basin*. Final Report. 2010. Tacoma, WA.

Fisheries Analysis

Fisheries benefits and costs are being studied under a separate contract with the Flood Authority by Anchor QEA. The results of that effort will not be known until mid-2011, and are not included in this report.

Regional Analysis

Because state and other regional funding decisions may be based on transfers in and out of the region, a regional analysis is provided. This regional analysis includes the monetized environmental benefits and avoided personal and business income losses. EESC used FEMA's HAZUS model estimate of the value of economic activity disrupted during flood events.

Benefits and Costs Estimated Using Corps Methodology

Introduction

National Economic Development (NED) flood damages are estimated using a benefit-cost model developed by EESC based on the methodology described in the Corps' 2003 report, the *Principles & Guidelines* documents and with inputs from a FEMA developed model (HAZUS) which also follows the Corps' methodology. The Corps approves flood reduction projects based on the results of a benefit-cost analysis using its *Principles & Guidelines* methodology, which evaluates economic development costs and benefits from a national perspective. The project benefit is equal to the avoided flood damages resulting from building a project.

There are a number of different costs and damages included. The HAZUS model identifies damages to building, structure, and contents as well as land based on different levels of flood event. In addition, EESC incorporated damage estimates due to transportation delays, public assistance and emergency aid, and other items not included in the HAZUS model but used in the Corps methodology. Each retention structure is modeled separately in the benefit-cost analysis.

Relevant national benefits, as defined by the Corps' methodology, attributable to the proposed projects are described below:

National Benefits³²

- Reduced estimated annual damage to building structures and contents, agriculture crops and equipment
- Avoided clean-up costs
- Avoided transportation delays or detours
- Avoided infrastructure improvement or operation and maintenance
- Increased availability of water for irrigation or other use
- Value of hydropower and its renewable qualities
- Increased recreation visits

These values were monetized; non-monetized values are discussed later in this section.

Flood Damages

This section first describes the data used in the HAZUS flood model to estimate flood damages. Then, the calculation of each of the avoided damages listed above are provided to quantify the amount of benefits

³² Note that the proposed flood reduction projects are not intended to completely prevent flooding and the consequent damages under any and all circumstances.

Building and Content Damage

The HAZUS flood model uses damage functions to evaluate dollar value of building and content damages based on building type. A damage function relates flood depth (feet) and percent of damage. To estimate building-related damages, HAZUS uses either depth of flooding at a particular building or an area weighted depth throughout the census block. Census blocks might correspond to one city block or might encompass several acres of vacant land. Content value is based on depreciated building value. Table 6 summarizes default structure damage functions in the HAZUS model. USACE-Galveston damage curves are generally used to determine flood damages if no local data is available.³³

³³ While calibration of the Corps and HAZUS damage prediction models against local conditions could be desirable, this is beyond the scope of this Phase IIB Feasibility Study.

Table 6
HAZUS Damage Functions – Structure Damage

Occupancy Class	Flooding Type/Zone	Curve Source	Curve Description
RES 1	Riverine/A-zone	FIA "credibility-weighted" depth damage curves (CWDD)	1 floor, no basement
	Riverine/A-zone	FIA CWDD	2 floors no basement
	Riverine/A-zone	FIA CWDD	2 floors, split level, no basement
	Riverine/A-zone	Modified FIA CWDD	EQE-modified versions of FIA CWDD
	Riverine/A-zone	Modified FIA CWDD	2 floors, with basement
	Riverine/A-zone	Modified FIA CWDD	2 floors, split level, with basement
	Coastal/V-zone	FIA V-zone damage function	Combined curve (average with and without obstruction)
	Coastal/V-zone	FIA V-zone damage function	Combined curve (average with and without obstruction)
RES2	All Zones	FIA CWDD	Mobile home
RES3	All Zones	USACE-Galveston ³⁴	Apartment
RES4	All Zones	USACE-Galveston	Average of hotel and motel unit
RES6	All Zones	USACE-Galveston	Nursing Home
COM1	All Zones	USACE-Galveston	Average of 47 retail classes
COM2	All Zones	USACE-Galveston	Average of 22 wholesale/warehouse classes
COM3	All Zones	USACE-Galveston	Average of 16 personal and repair services classes
COM4	All Zones	USACE-Galveston	Average of business and office
COM5	All Zones	USACE-Galveston	Bank
COM6	All Zones	USACE-Galveston	Hospital
COM7	All Zones	USACE-Galveston	Average of 4 medical office/clinic classes
COM8	All Zones	USACE-Galveston	Average of 15 entertainment and recreation classes

Source: Scawthorn, Charles et al. *HAZUS-MH Flood Loss Estimation Methodology. II. Damage and Loss Assessment*. Natural Hazards Review. May 2006.

³⁴ Galveston curves are used since a significant amount of data exists for that location. Corps analyses commonly use these curves for analyses in other parts of the country.

Similarly, Table 7 illustrates the default damage functions for building content.

Table 7			
HAZUS Damage Functions - Content Damages			
Occupancy Class	Flooding Type/Zone	Curve Source	Curve Description
RES 1	Riverine/A-zone	FIA "credibility-weighted" depth damage curves (CWDD)	Residential Contents, 1st Floor Only (1 floor, no basement)
	Riverine/A-zone	FIA "credibility-weighted" depth damage curves (CWDD)	Residential Contents – 1st floor and Above (2 floors no basement and 2 floor, split level, no basement)
	Riverine/A-zone	Modified FIA CWDD	EQE-modified versions of FIA CWDD Residential contents - 1st floor and above (and 2 floor with basement and 2 floor split level with basement)
	Coastal/V-zone	FIA V-zone damage function	Combined curve (average with and without obstruction)
RES2	All Zones	FIA CWDD	Contents - Mobile home
RES3	All Zones	USACE-Galveston	Contents - Apartment
RES4	All Zones	USACE-Galveston	Average of hotel-equipment and motel unit-inventory
RES6	All Zones	USACE-Galveston	Nursing Home - equipment
COM1	All Zones	USACE-Galveston	Average of 47 retail classes - equipment and inventory, when available

Source: Scawthorn, Charles et al. *HAZUS-MH Flood Loss Estimation Methodology. II. Damage and Loss Assessment*. Natural Hazards Review. May 2006.

Residential Structure and Content Damages

The HAZUS flood damage estimate model includes home and structure inventories in census block levels.³⁵ The model combines depth damage functions and depreciated home values to obtain structure and content damages. Depreciated home values are determined using Dunn & Bradstreet data; the Corps used this method to depreciate property value in its 2003 Report. Dunn & Bradstreet data allow residential properties to be depreciated according to building age, structure type, and condition. Depth damage functions are either from the Federal Insurance Administration or the U.S. Army Corps of Engineers (see Tables 6 and 7).

HAZUS estimates that the depreciated dollar value of exposed residential buildings is \$694 million dollars. Table 8 summarizes damages to residential buildings and content for the four flood events (10, 50, 100, 500 years) with and without the projects.

³⁵ 2000 Census data used with value updated to 2006 values according to Means data/methodology in HAZUS. EESC escalated costs to 2010 values.

Table 8
HAZUS Output: Residential Building and Content Benefit Calculation
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
<i>Structure Damages</i>				
Base	\$31,609,282	\$49,135,432	\$64,527,125	\$161,038,940
With Upper Chehalis Project	\$26,568,100	\$38,227,173	\$44,405,302	\$79,350,345
Upper Chehalis Project Benefit	\$5,041,182	\$10,908,259	\$20,121,823	\$81,688,595
With South Fork Chehalis Structure	\$30,075,475	\$45,241,924	\$55,656,791	\$133,998,899
South Fork Chehalis Project Benefit	\$1,533,806	\$3,893,508	\$8,870,335	\$27,040,040
With Both Projects	\$24,969,938	\$35,277,546	\$40,286,550	\$63,980,104
Benefit from Both Projects	\$6,639,344	\$13,857,887	\$24,240,576	\$97,058,836
<i>Content Damages</i>				
Base	\$21,087,156	\$33,035,829	\$45,434,991	\$116,054,438
With Upper Chehalis Project	\$17,762,121	\$25,645,671	\$29,753,698	\$56,514,864
Upper Chehalis Project Benefit	\$3,325,035	\$7,390,158	\$15,681,293	\$59,539,573
With South Fork Chehalis Structure	\$20,003,838	\$30,247,090	\$38,066,285	\$56,514,864
South Fork Chehalis Project Benefit	\$1,083,318	\$2,788,739	\$7,368,706	\$59,539,573
With Both Projects	\$16,732,433	\$23,693,554	\$26,964,959	\$45,081,035
Benefit from Both Projects	\$4,354,723	\$9,342,275	\$18,470,032	\$70,973,402

Commercial and Industrial Structure and Content Damages

Commercial and industrial buildings are valued based on observed age and building frame material. The HAZUS model estimates that the depreciated dollar value of exposed commercial and industrial buildings is approximately \$270 million dollars.

Tables 9 and 10 show commercial and industrial building and content damages from the HAZUS model.

Table 9
HAZUS Output: Commercial Structure, Content, and Inventory Damages for Benefit Calculation
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
<i>Structure Damages</i>				
Base	\$14,147,486	\$27,029,314	\$34,934,316	\$59,668,284
With Upper Chehalis Project	\$10,296,882	\$19,456,816	\$23,425,406	\$39,642,995
Upper Chehalis Project Benefit	\$3,850,605	\$7,572,498	\$11,508,910	\$20,025,290
With South Fork Chehalis Structure	\$13,064,169	\$23,532,665	\$30,440,157	\$53,264,911
South Fork Chehalis Project Benefit	\$1,083,318	\$3,496,649	\$4,494,160	\$6,403,373
With Both Projects	\$9,374,453	\$16,925,499	\$20,894,089	\$34,237,132
Benefit from Both Projects	\$4,773,034	\$10,103,815	\$14,040,227	\$25,431,153
<i>Content Damages</i>				
Base	\$39,171,054	\$72,582,290	\$92,682,661	\$152,586,916
With Upper Chehalis Project	\$28,573,847	\$53,104,022	\$63,122,030	\$103,558,743
Upper Chehalis Project Benefit	\$10,597,207	\$19,478,268	\$29,560,631	\$49,028,173
With South Fork Chehalis Structure	\$36,242,878	\$63,465,259	\$81,152,299	\$103,558,743
South Fork Chehalis Project Benefit	\$2,928,176	\$9,117,031	\$11,530,362	\$49,028,173
With Both Projects	\$26,139,063	\$46,250,160	\$56,772,286	\$90,859,255
Benefit from Both Projects	\$13,031,991	\$26,332,130	\$35,910,375	\$61,727,661
<i>Inventory Loss</i>				
Base	\$1,405,095	\$2,424,058	\$2,960,353	\$5,030,456
With Upper Chehalis Project	\$1,051,140	\$1,909,213	\$2,166,636	\$3,249,953
Upper Chehalis Project Benefit	\$353,955	\$514,844	\$793,718	\$1,780,502
With South Fork Chehalis Structure	\$1,297,836	\$2,177,361	\$2,660,028	\$4,483,434
South Fork Chehalis Project Benefit	\$107,259	\$246,696	\$300,326	\$547,022
With Both Projects	\$986,784	\$1,683,969	\$1,995,021	\$2,917,450
Benefit from Both Projects	\$418,311	\$740,088	\$965,333	\$2,113,006

Table 10
HAZUS Output: Industrial Structure, Content, and Inventory Damages for Benefit Calculation
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
<i>Structure Damages</i>				
Base	\$5,985,062	\$9,363,727	\$11,551,814	\$18,223,335
With Upper Chehalis Project	\$4,579,967	\$7,443,787	\$8,645,090	\$13,257,235
Upper Chehalis Project Benefit	\$1,405,095	\$1,919,939	\$2,906,724	\$4,966,100
With South Fork Chehalis Structure	\$5,588,204	\$8,548,557	\$10,275,430	\$16,764,611
South Fork Chehalis Project Benefit	\$396,859	\$815,170	\$1,276,384	\$1,458,725
With Both Projects	\$4,150,930	\$6,703,699	\$7,926,454	\$11,508,910
Benefit from Both Projects	\$1,834,132	\$2,660,028	\$3,625,360	\$6,714,425
<i>Content Damages</i>				
Base	\$19,371,009	\$29,753,698	\$36,328,686	\$53,833,385
With Upper Chehalis Project	\$14,833,945	\$24,036,783	\$27,436,899	\$41,638,015
Upper Chehalis Project Benefit	\$4,537,064	\$5,716,915	\$8,891,786	\$12,195,369
With South Fork Chehalis Structure	\$18,094,624	\$27,222,381	\$32,789,133	\$41,638,015
South Fork Chehalis Project Benefit	\$1,276,384	\$2,531,317	\$3,539,553	\$12,195,369
With Both Projects	\$13,589,739	\$21,923,777	\$25,420,427	\$36,339,412
Benefit from Both Projects	\$5,781,270	\$7,829,920	\$10,908,259	\$17,493,973
<i>Inventory Loss</i>				
Base	\$3,174,872	\$4,901,745	\$5,931,433	\$8,859,609
With Upper Chehalis Project	\$2,413,332	\$3,947,138	\$4,504,886	\$6,768,055
Upper Chehalis Project Benefit	\$761,540	\$954,607	\$1,426,547	\$2,091,554
With South Fork Chehalis Structure	\$2,949,628	\$4,472,708	\$5,395,137	\$8,323,313
South Fork Chehalis Project Benefit	\$225,244	\$429,037	\$536,296	\$536,296
With Both Projects	\$2,220,265	\$3,603,909	\$4,183,108	\$5,942,159
Benefit from Both Projects	\$954,607	\$1,297,836	\$1,748,325	\$2,917,450

Other

The HAZUS model estimates building and content damages for several categories grouped under “Other.” The “Other” category includes damages to government properties, schools, agriculture, and religious buildings. Table 11 summarizes flood damages to buildings and contents in the “Other” category.

Table 11
HAZUS Output: Other Structure, Content, and Inventory Damages for Benefit Calculation
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
<i>Structure Damages</i>				
Base	\$986,784	\$1,887,762	\$2,563,495	\$5,663,285
With Upper Chehalis Project	\$793,718	\$1,287,110	\$1,576,710	\$3,174,872
Upper Chehalis Project Benefit	\$193,067	\$600,651	\$986,784	\$2,488,413
With South Fork Chehalis Structure	\$922,429	\$1,598,162	\$2,188,087	\$4,826,663
South Fork Chehalis Project Benefit	\$64,356	\$289,600	\$375,407	\$836,622
With Both Projects	\$729,362	\$1,072,592	\$1,287,110	\$2,466,961
Benefit from Both Projects	\$257,422	\$815,170	\$1,276,384	\$3,196,324
<i>Content Damages</i>				
Base	\$3,861,331	\$7,143,462	\$9,878,571	\$24,412,190
With Upper Chehalis Project	\$3,260,679	\$4,719,404	\$5,985,062	\$12,720,939
Upper Chehalis Project Benefit	\$600,651	\$2,424,058	\$3,893,508	\$11,691,251
With South Fork Chehalis Structure	\$3,668,264	\$5,942,159	\$8,258,957	\$12,720,939
South Fork Chehalis Project Benefit	\$193,067	\$1,201,303	\$1,619,614	\$11,691,251
With Both Projects	\$3,046,161	\$4,204,560	\$5,019,730	\$9,749,860
Benefit from Both Projects	\$815,170	\$2,938,902	\$4,858,841	\$14,662,331
<i>Inventory Loss</i>				
Base	\$150,163	\$311,052	\$364,681	\$622,103
With Upper Chehalis Project	\$128,711	\$182,341	\$257,422	\$407,585
Upper Chehalis Project Benefit	\$21,452	\$128,711	\$107,259	\$214,518
With South Fork Chehalis Structure	\$139,437	\$278,874	\$343,229	\$579,200
South Fork Chehalis Project Benefit	\$10,726	\$32,178	\$21,452	\$42,904
With Both Projects	\$117,985	\$160,889	\$193,067	\$343,229
Benefit from Both Projects	\$32,178	\$150,163	\$171,615	\$278,874

Agriculture Crops

Damage to agriculture crops is estimated using a combination of HAZUS output and dollars updated from the 2003 Corps report. The HAZUS model estimates the number of agricultural acres flooded during an event. Crop damage per acre is valued as a weighted average between hay, green peas, and sweet corn.³⁶ These are the three primary crops produced in Lewis County according to the USDA published data for 2000 to 2008. Table 12 summarizes the value per

³⁶ Weighting is updated from the 2003 Corps report.

acre and the share of acreage for each crop type. The crop values are taken from the 2003 Corps report and escalated to 2010 dollars using the GDP deflator.

Table 12
Crop Value, 2010 Dollars

	\$/Acre	Share
Hay	\$276.27	86%
Green Peas	\$62.15	6%
Sweet Corn	\$72.55	8%
Weighted Average	\$247.13	

The value per acre is applied to the number of acres flooded with and without projects. Table 13 shows the number of acres flooded with and without each project.

Table 13
HAZUS Output: Flooded Acres of Agriculture

	10-Year	50-Year	100-Year	500-Year
Base	10,870	14,052	14,965	16,392
Upper Chehalis Structure	9,579	12,232	13,201	14,919
South Fork Chehalis Structure	10,405	13,428	14,611	16,087
Both Structures	9,174	11,246	12,249	14,354

Table 14 summarizes costs to agriculture using crop value from Table 11 and the number of flooded acres estimated by HAZUS (Table 12). Clean-up and field restoration costs are estimated under “Clean-Up Costs” in this section.

Table 14
Agriculture Crop Damage Estimates
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
Base	\$2,686,283	\$3,472,644	\$3,698,272	\$4,050,924
With Upper Chehalis Structure	\$2,367,240	\$3,022,871	\$3,262,338	\$3,686,904
Upper Chehalis Project Benefit	\$319,042	\$449,773	\$435,934	\$364,020
With South Fork Chehalis Structure	\$2,571,368	\$3,318,436	\$3,610,789	\$3,975,550
South Fork Chehalis Project Benefit	\$114,915	\$154,208	\$87,483	\$75,374
With Both Projects	\$2,267,153	\$2,779,203	\$3,027,072	\$3,547,277
Benefit from Both Projects	\$419,129	\$693,441	\$671,200	\$503,647

Not valued in the above analysis is the potential for farmers to plant some of the protected fields earlier in the with-project cases. Due to the flood protection in the winter and spring, local farm land may be more productive creating a value from a national perspective. Given that some farmland may experience increased productivity as a result of the flood retention structures, benefits calculated here may be underestimated.

Also not valued in the above analysis, are the costs and benefits to agriculture attributable to flooding such as soil renewal or groundwater recharge. The nature and value of such benefits, are not estimated and require further independent study once more is known regarding the operational characteristics of the structures. Some flooding would occur after project completion, albeit at reduced frequency and severity.

Emergency Costs

Emergency aid includes Temporary Relocation Assistance and Public Assistance expenditures from FEMA. To calculate these damages, the Corps’ methodology averages emergency expenditures across several locations in the U.S. The resulting value of \$1,537 per claim is used in the 2003 Study. In addition, twenty-five percent contributions from state or local governments may be added to the emergency costs. The Corp determined the relationship of Temporary Relocation Assistance to Public Assistance (3.01 in the 2003 Study) and applied that value to each claim.

For this study, historic emergency costs were used to determine avoided damages. In addition, the HAZUS model estimated shelter needs for displaced individuals as well as damages to essential facilities such as hospitals, fire stations, and schools. Both the HAZUS model and historic data were used to determine the best estimate of emergency costs for various flood return intervals.

Table 15 summarizes relocation costs for three flood events calculated using the number of households displaced from the HAZUS output and Corps estimates for Temporary Relocation Assistance and Emergency Repair claims. The 2003 Corps report found that the average claim for Temporary Relocation Assistance or Emergency Repairs so residents could stay in their homes is \$1,812 (2010 dollars). The analysis assumed one claim per household.

Table 15 HAZUS Output: Relocation (Temporary Relocation Assistance) and Emergency Repair Benefits 2010 Dollars				
	10-Year	50-Year	100-Year	500-Year
Base	\$1,395,707	\$2,099,897	\$3,023,128	\$7,239,214
With Upper Chehalis Structure	\$1,171,236	\$1,665,436	\$1,895,338	\$4,033,251
Upper Chehalis Project Benefit	\$224,472	\$434,461	\$1,127,790	\$3,205,964
With South Fork Chehalis Structure	\$1,307,005	\$1,933,354	\$2,465,569	\$6,647,260
South Fork Chehalis Project Benefit	\$88,703	\$166,544	\$557,559	\$591,954
With Both Projects	\$1,122,359	\$1,516,995	\$1,741,466	\$3,050,282
Benefit from Both Projects	\$273,349	\$582,902	\$1,281,661	\$4,188,933

In addition to Temporary Relocation Assistance and Emergency Repair costs, the Corps calculates Public Assistance funds required. Public Assistance funds are calculated as a ratio of Public Assistance to Temporary Relocation Assistance. The 2003 Corps report used a ratio of 3.01 (Public Assistance to Temporary Relocation Assistance) plus an additional 25 percent³⁷ for state and local contributions. Table 16 summarizes Public Assistance funds required under each flood event.

Table 16 Public Assistance Benefits 2010 Dollars				
	10-Year	50-Year	100-Year	500-Year
Base	\$5,251,349	\$7,900,863	\$11,374,518	\$27,237,543
With Upper Chehalis Structure	\$4,406,774	\$6,266,201	\$7,131,210	\$15,175,105
Upper Chehalis Project Benefit	\$844,575	\$1,634,661	\$4,243,308	\$12,062,438
With South Fork Chehalis Structure	\$4,917,606	\$7,274,243	\$9,276,703	\$25,010,317
South Fork Chehalis Project Benefit	\$333,743	\$626,620	\$2,097,815	\$2,227,226
With Both Projects	\$4,222,875	\$5,707,692	\$6,552,267	\$11,476,684
Benefit from Both Projects	\$1,028,474	\$2,193,171	\$4,822,251	\$15,760,859

Clean-Up Costs

In their 2003 report, the Corps estimates clean-up costs as part of the structure, content, and emergency aid provided during the 2007 flood event. This Phase IIB analysis uses the Corps' methodology of assigning a \$/sq foot or \$/acre for building and agriculture clean-up costs. There are three main calculations that go into the clean-up cost estimation. These calculations include the following:

1. Debris removal
2. Building clean-up costs (commercial and residential)
3. Agriculture field clean-up and restoration

First, the debris removal cost were calculated from the HAZUS model output. HAZUS estimates the tons of debris and the number of truckloads required to remove the debris. A typical tipping fee for debris removal is \$82/ton. At 25 tons per truckload, one truckload costs \$2,050 (2010 dollars). The number of truckloads in the base case ranged from 447 to 2,423 from the 10-year event to the 500-year event. Table 17 summarizes the debris removal cost calculation.

³⁷ While the Corps uses a relationship consistent with an additional 15% for local funds, up to 25% is allowed by *Principles & Guidelines* methodology. Reference Table 13 in Appendix D: Economics of the June 2003 Corps Report.

Table 17
Debris Removal, Truckloads

	10-Year	50-Year	100-Year	500-Year	<i>formula</i>
Base	447	710	1,026	2,423	<i>a</i>
With Upper Chehalis Structure	359	541	629	1271	<i>b</i>
Upper Chehalis Project Benefit	\$180,400	\$346,450	\$813,850	\$2,361,600	$c = (a - b) \times \$625$
With South Fork Chehalis Structure	422	640	811	2073	<i>d</i>
South Fork Chehalis Project Benefit	\$51,250	\$143,500	\$440,750	\$717,500	$e = (a - d) \times \$625$
With Both Projects	338	496	567	992	<i>f</i>
Benefit from Both Projects	\$223,450	\$438,700	\$940,950	\$2,933,550	$g = (a - f) \times \$625$

Second, labor costs to clean up floodwaters, dry out, and decontaminate structures and content were included under building clean-up. These costs are based on the \$4.30/square foot provided in the 2003 Corps report (\$4.30 is \$3.65 adjusted to 2010 dollars). The number of buildings with damage from HAZUS, and average building size,³⁸ are used to calculate these additional clean-up costs for all building types. Table 18 summarizes the results of the building clean-up cost calculations. See Table A-1 in Appendix A for more detail on number of buildings and specific calculations.

Table 18
Building Clean-Up Costs Summary

	10-Year	50-Year	100-Year	500-Year
Base	\$1,567,385	\$3,270,616	\$4,418,426	\$11,747,219
With Upper Chehalis Structure	\$1,244,966	\$2,252,632	\$2,813,211	\$5,739,054
Upper Chehalis Project Benefit	\$322,419	\$1,017,983	\$1,605,215	\$6,008,166
With South Fork Chehalis Structure	\$1,457,333	\$2,785,698	\$3,683,312	\$10,732,675
South Fork Chehalis Project Benefit	\$110,052	\$484,918	\$735,115	\$1,014,544
With Both Projects	\$1,203,697	\$1,992,118	\$2,409,973	\$4,323,850
Benefit from Both Projects	\$363,688	\$1,278,498	\$2,008,454	\$7,423,369

Finally, agriculture clean-up costs were calculated using the number of acres flooded (HAZUS output) and a dollar per-acre unit cost. The Corps estimated in their 2003 report that clean-up costs and field restoration are approximately \$286/acre for agriculture (2010 dollars). Table 19 summarizes the costs for field restoration and clean-up.

³⁸ Average commercial building size is 15,800 square feet, while residential buildings are 1,600 square feet on average, from Lewis County PUD's 2009 Conservation Potential Study.

Table 19
Agriculture Field Restoration and Clean-Up Costs

	10-Year	50-Year	100-Year	500-Year
Base	\$2,686,283	\$3,472,644	\$3,698,272	\$4,050,924
With Upper Chehalis Structure				
Upper Chehalis Project Benefit	\$2,367,240	\$3,022,871	\$3,262,338	\$3,686,904
With South Fork Chehalis Structure	\$319,042	\$449,773	\$435,934	\$364,020
South Fork Chehalis Project Benefit	\$2,571,368	\$3,318,436	\$3,610,789	\$3,975,550
With Both Projects	\$114,915	\$154,208	\$87,483	\$75,374
Benefit from Both Projects	\$2,267,153	\$2,779,203	\$3,027,072	\$3,547,277

Table 20 summarizes the total estimated clean-up costs for debris disposal, building clean-up, and agriculture field clean-up and restoration.

Table 20
Clean-Up Costs
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
Base	\$5,594,749	\$8,747,824	\$10,804,737	\$21,405,790
With Upper Chehalis Structure	\$4,722,444	\$6,862,503	\$7,880,812	\$12,614,449
Upper Chehalis Project Benefit	\$872,305	\$1,885,321	\$2,923,925	\$8,791,341
With South Fork Chehalis Structure	\$5,300,363	\$7,940,816	\$9,527,557	\$19,586,454
South Fork Chehalis Project Benefit	\$294,386	\$807,008	\$1,277,180	\$1,819,336
With Both Projects	\$4,522,212	\$6,227,544	\$7,078,009	\$10,465,592
Benefit from Both Projects	\$1,072,537	\$2,520,280	\$3,726,728	\$10,940,198

Transportation

Interstate 5 Closure

During major flood events, Interstate 5 (I-5) is usually closed for multiple days. When the freeway is closed, significant losses are incurred at the national and local level. These losses include the increased cost for transporting goods between the Seattle and Portland metropolitan areas. Detours for cars and freight trucks impose significant costs especially since winter road conditions extend the Seattle-Portland trip by 440 miles. Because of the significant cost of closing major North-South route, WSDOT has estimated the cost to raise I-5 so that the freeway can remain open during flooding events. The benefits to the proposed flood retention structures includes the avoided days of closure in the short term (detour costs), and the avoided cost of having to raise I-5 in the longer-term. Avoided detour costs are considered for the first ten years

of the analysis, while avoided costs for raising the I-5 roadway level are included as benefits after year 10.

In general, I-5 transportation delay and detour costs are estimated using cost per vehicle separated into cars (and light trucks) and freight traffic. The methodology used to estimate both the number of trips and the value of trips for cars is taken directly from the Corps' 2003 report. This methodology accounts only for current traffic volumes (no escalation due to economic or population growth). The value of each trip is calculated using updated assumptions from the Corp's 2003 analysis as well. These assumptions include the median household income, detour route, vehicle mileage costs, and occupancy. Future studies could utilize more sophisticated and detailed modeling of traffic to account for different types of trips (business, personal, charity) and to account for the value of trips that do not occur when that value differs across purpose and duration of closure. In particular, WSDOT has a traffic model for Lewis County that includes I-5 information. As more detailed studies are conducted, analysts could utilize the traffic model and its data to more accurately estimate the cost of I-5 closure on car traffic.

The section below first discusses the methodology used to value the flood reduction benefits to I-5 of each with project scenario. Following the methodology description, the cost estimates are given.

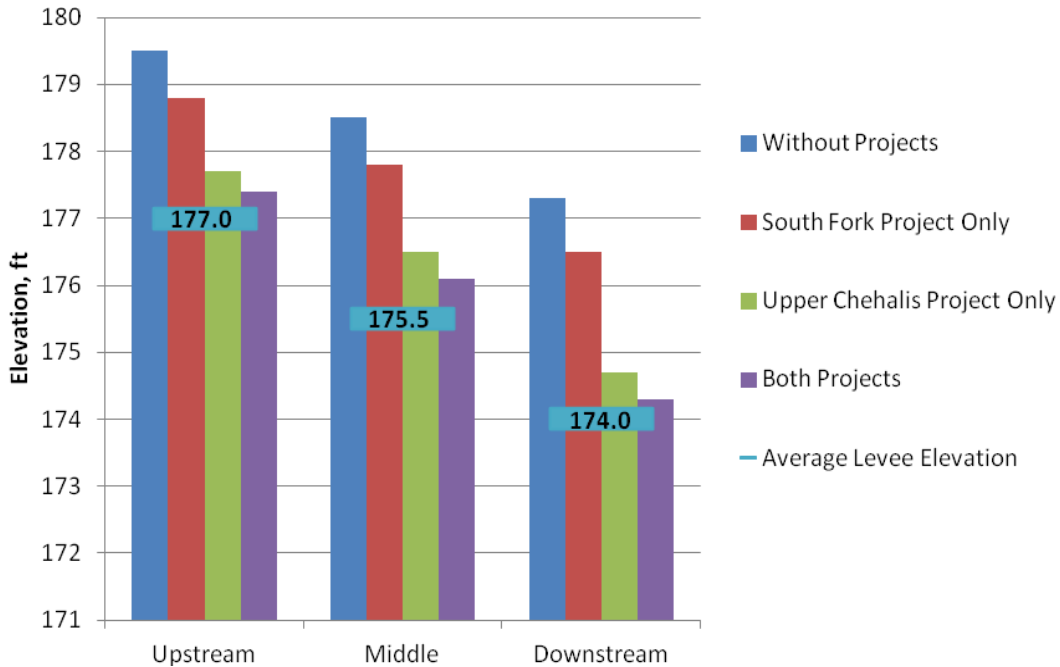
Avoided Cost to Raise I-5

The key assumption made for the analysis is that WSDOT would not raise the freeway if I-5 stays dry in the 100-year flood, thereby avoiding a cost of approximately \$100 million. Historically, I-5 has been flooded in three main areas: the Highway 6 overpass, near the airport, and Salzer Creek to Mellen Street. Northwest Hydraulic Consultants modeled several flood events for with- and without project scenarios. From their modeling, the area of I-5 near the airport was the only concern.³⁹

In past flood events, water has overtopped the airport levee creating pools on the other side and over I-5. Even when flood waters recede, the pool behind the airport levee is slow to drain since the water is trapped behind the levee. In 2009, the levee was temporarily breached to allow flood waters to drain back into the river. NHC analyzed flood elevations near the airport levee for each with and without project scenario. Figure 11 compares the average levee elevation and flood levels for a 100 year event.

³⁹ The following caveats should be considered with regard to the data provided by NHC. The existing levee elevations are average; there are high and low spots along the reaches modeled. The flood events modeled refer to flows consistent with the flood return interval events on the Chehalis River. In addition, significant events on tributaries are not considered. Finally, while the hydrology model is calibrated to the 100 year events, the model is not calibrated for the 10, 50 and 500-year events.

Figure 11
Flood Elevations and Levee Elevations near the Chehalis Airport, 100-Year Event



From Figure 11 above, the hydrology model shows that in the 100-year event, the levee is overtopped in various places even with storage on both the Upper Chehalis and South Fork reaches. While the airport levee overtops in the with-project scenarios, the amount by which the levee is overtopped is significantly reduced. Therefore, project implementation reduces the cost to raise the levee to the 100-year level of protection.

Value of Reduced Flood Level to I-5

In two with-project scenarios (Upper Chehalis Only and Both Projects), the value of reduced flood levels is estimated by accounting for the cost of levee improvements needed to keep I-5 dry in the 100-year event. The full avoided cost to raise I-5 is included as a benefit (\$100 million); however, the cost to raise the airport levee is included on the cost side of the benefit-cost ratio. This methodology credits the projects with benefits that would otherwise be excluded given the hydrology model estimates.

Cost to Raise Levee

The cost to raise the airport levee is estimated based on the costs developed in the Corps’ 2003 General Reevaluation study of the Chehalis River Basin. In their 2003 report, the Corps estimates that all Chehalis River levees (both new and new on top of existing levees) would cost approximately \$47.8 million (escalated to 2010 dollars). The costs for each individual levee are not provided in the report, so the total cost is allocated based on the amount of fill used for each proposed levee/modification. The fill to raise the airport levee and to continue the levee in both the north and south direction is approximately 29 percent of the total cost, or \$13.6 million. This

allocation method is likely to overestimate the costs of the levee improvements for the following reasons:

1. The extensions to the airport levee are included in the estimate; however, they may not be needed with flood storage;
2. Some modifications/new levees on the Chehalis River do not require fill and therefore are allocated 0 percent of the total cost. (The 29% estimated share of cost to the airport levee project may be too high);
3. The average height increase to the airport levee is 1.5 feet; however, Figure 11 shows that the average height increase needed is only 0.8 feet in the Upper Chehalis Only case and 0.4 feet in the Both Projects case.

It is assumed that \$13.6 million will allow for airport levee modifications needed for 100-year projection in the two applicable with-project cases (Upper Chehalis Only and Both Projects). In the without project cases, the levee would need to be raised an average 3 feet to provide protection to I-5 from a 100-year flood event. In addition, under the without project scenario, the Highway 6 overpass and the area between Salzer Creek and Mellen Street would still flood.

Traffic Detour Costs

In its 2003 report, the Corps estimated that during a 100-year event, I-5 is closed for 4.5 days. For that period, traffic is re-routed on a 101-mile detour that takes 3.13 hours in additional travel time. The Corps used Washington State Department of Transportation (WSDOT) annual traffic reports to determine the amount of normal through traffic. The number of vehicles was broken out by type (car vs. heavy truck) and assigned a value for time and mileage. For this report, the detour costs for cars are estimated using this same method. Detour costs for freight trucks is estimated according to detour and cost estimates developed by WSDOT after the 2007 flood event.

Cost to Car Traffic

In 2009, there were 58,000 average daily vehicles at milepost 86.32. This is the same amount of traffic reported for 2008. It is believed that 2008 was a peak traffic year. More recent traffic data may indicate lower average daily traffic; however, since the study was over a 50-year period, and traffic volumes are not escalated over the period, 58,000 vehicles per day is representative of the projected average over the period. Nineteen percent of these vehicles are classified as trucks. Table 21 summarizes the data and sources for the estimated damages due to I-5 closure.

Table 21
Interstate 5 Closure Assumptions for Damage Estimates
2010 Dollars

		Source:
Average Daily Thru Traffic	58,000	<i>WSDOT 2009 Annual Traffic Report</i>
Percent Trucks	19%	<i>WSDOT 2009 Annual Traffic Report</i>
Trucks	11,020	$= 58,000 \times 0.19$
Cars	46,980	$= 58,000 \times (1 - 0.19)$
Median Family Income, 2009 Projection	\$38,319	<i>WA Office of Financial Management</i>
Average Hourly Rate, 2009	\$18.42	40 hours per week, 52 weeks
Value of Time (VOT) (53.8% of hourly wage)	\$9.91	<i>Corps' 2003 report</i>
Vehicle Operation Costs – Car, \$/mile	\$0.500	<i>IRS, 2010</i>
Diversion, miles	101	<i>Corps' 2003 report</i>
Velocity, MPH	32	<i>Corps' 2003 report</i>
Hours	3.16	<i>Corps' 2003 report</i>
Occupancy Factor, Car	1.15	<i>Corps' 2003 report</i>

The calculation for daily costs for car traffic is shown in Table 22.

Table 22
Daily Cost Due to I-5 Closure
2010 Dollars

VOT, \$/hour	\$9.91	<i>a</i>
Occupancy Factor	1.15	<i>b</i>
Occupancy-Weighted VOT	\$11.40	$c = a \times b$
Detour Travel Time, hours	3.16	<i>d</i>
Time Costs	\$35.98	$e = c \times d$
Diversion Miles	101	<i>f</i>
Vehicle Operating Costs, \$/mile	\$0.50	<i>g</i>
Diversion Mileage Cost	\$50.50	$h = f \times g$
Total Cost per Vehicle	\$86.48	$i = e + h$
Vehicle Units	46,980	<i>j</i>
Daily Time Costs	\$1,690,111	$k = e \times j$
Daily Mileage Costs	\$2,372,490	$l = h \times j$
Total Daily Cost	\$4,062,601	$m = k + l$

Cost to Freight Traffic

The cost for transportation detours to freight trucks is calculated separately by applying the Corps' methodology to detour data presented in the WSDOT 2008 report regarding freeway

closures in 2007.⁴⁰ According to the report, freight companies reported that the detour routes cost an additional \$500 to \$850 per truckload. The cost per truck used in this analysis is calculated from a weighted average based on the number of trips taken for each detour route during the 2007 closure. The number of trips per detour route was estimated based on the actual traffic volumes during the week of the 2007 flood compared to the same week the previous year. The resulting weighted average is \$551 per truck. These costs take into consideration additional time, fuel, mileage, and the 10-hour rest period required for drivers after 11 hours of driving. The four detour routes below were considered.

- I-84 in Oregon, over I-82 and I-90 in Washington (additional 440 miles) – cost: \$800-\$850/truckload
- US 97 to I-82 and I-90 in Washington (344 additional miles) – Cost is \$500/truckload
- US 97 to US 12 to US 7 – Cost is \$500/truckload
- US 12 to US 7 (85 additional miles, certain trucks allowed on a case-by-case basis) – Cost is \$100/truckload

These routes are illustrated in Figure 12.

Figure 12
I-5 Truck Detour Routes



⁴⁰ WSDOT. *Storm-Related Closures of I-5 and I-90: Freight Transportation Economic Impact Assessment Report*. Winter 2007-2008. Final Research Report September 2008.

Based on WSDOT heavy truck counts for the closure period in 2007, the average daily truck volume is distributed among these routes to produce a weighted average cost for each diverted truckload. According to actual traffic reports, most trucks took the US 97 to I-82 and I-90 detour. The weighted average cost for all detours is approximately \$551/truckload (2010 dollars). Total cost per day for freight truck detours is \$6.1 million. These costs do not include regional economic impacts.

Table 23 summarizes the days of closure under the four flood return intervals with and without projects. The number of days I-5 is closed is determined from hydrology data provided by NHC and the assumption that the airport levee is modified.

Table 23
Interstate 5 Closure by Flood Return Interval

	10-Year	50-Year	100-Year	500-Year
Base				
Days of Closure	0	5.8	6.4	6.4
Damage, \$	\$0	\$58,801,258	\$60,828,887	\$64,884,147
With Upper Chehalis Structure				
Days of Closure	0	0	0	5.1
Damage, \$	\$0	\$0	\$0	\$51,704,554
Upper Chehalis Project Benefit	\$0	\$58,801,258	\$60,828,887	\$13,179,592
With South Fork Chehalis Structure				
Days of Closure	0	5.2	6.3	6.3
Damage, \$	\$0	\$52,718,369	\$59,815,073	\$63,870,332
South Fork Chehalis Project Benefit	\$0	\$6,082,889	\$1,013,815	\$1,013,815
With Both Projects				
Days of Closure	0	0	0	5.1
Damage, \$	\$0	\$0	\$0	\$51,704,554
Benefit from Both Projects	\$0	\$58,801,258	\$60,828,887	\$13,179,592

Railway Closure

Similar to I-5, the railway is usually closed during flood events. The number of days closure is estimated based on NHC's hydrology model for that area.⁴¹ The methodology for estimating

⁴¹ The following caveats should be considered with regard to the days of closure data provided by NHC. The elevations of existing levees in the model are approximate and represent average conditions; these were not determined by detailed field survey and as such may be different from actual conditions. Further there may be high and low spots along the reaches modeled that are not represented in the levee crest data. Secondly, the return periods associated with the modeled flood events refer to flows on the main stem of the Chehalis River. These represent general, basin wide flood events and as such do not consider large flood events on any particular tributary. Lastly, the baseline hydrologic data used was developed for the FEMA flood insurance study and was focused on estimated of the 100-year flood. The data developed for other return periods (10, 50, and 500-year events) were

cost per day of closure is consistent with the Corps' 2003 report. The Corps calculates operation costs and applies those to the additional miles required via the diversion route. Daily costs include equipment expense (\$27.44/car from the Corps' 2003 report), cost per mile (\$0.47/mile from the Corps' 2003 report), miles of diversion (350 from the Corps' 2003 report), and average daily through traffic (3,391 cars).⁴² Daily costs are calculated as shown in the equation below.

$$\text{Cost/day} = \text{daily through traffic} \times (\text{cost per mile} \times \text{miles} + \text{equipment expense})$$

The resulting cost for 1 day of railroad closure is \$652,161. This cost is applied to the days of days as shown in Table 24.

Table 24 Railway Closure Costs by Flood Return Interval				
	10-Year	50-Year	100-Year	500-Year
Base				
Days of Closure	1.4	2.2	2.3	2.5
Damage, \$	\$913,025	\$1,434,753	\$1,499,969	\$1,630,401
With Upper Chehalis Storage				
Days of Closure	0.9	1.6	1.7	2.1
Damage, \$	\$586,944	\$1,043,457	\$1,108,673	\$1,369,537
Upper Chehalis Project Benefit	\$326,080	\$391,296	\$391,296	\$260,864
With South Fork Chehalis Storage				
Days of Closure	1.3	1.9	2.1	2.2
Damage, \$	\$847,809	\$1,239,105	\$1,369,537	\$1,434,753
South Fork Chehalis Project Benefit	\$65,216	\$195,648	\$130,432	\$195,648
With Both Projects				
Days of Closure	0.9	1.4	1.5	2
Damage, \$	\$586,944	\$913,025	\$978,241	\$1,304,321
Benefit from Both Projects	\$913,025	\$521,728	\$521,728	\$326,080

Multi-Purpose Project Benefits

In addition to the avoided costs and resulting benefits provided by the flood protection element of the projects, this section describes project benefits that would result from the multi-purpose projects. Special permission from Congress may be required to deem a project multi-purpose and

developed using more approximate methods. While these data represent the best estimate of these flows currently available, the data should be used with appropriate caution.

⁴² Assuming 49 trains per day and 69 cars per train based on Washington State Transportation Commission's Statewide Rail Capacity and Systems Needs Study, 2006.

have these benefits included in a benefit-cost analysis used to determine a preferred alternative for National Environmental Policy Act (NEPA) purposes.

Summer Flow Augmentation

The priority benefit of the multi-purpose project is the ability to augment summer flows. However, this kind of change to the environment cannot be monetized for inclusion in a National Economic Development type analysis. The potential benefit of summer flow augmentation will therefore be discussed in the Alternative Analysis section.

Hydroelectric Power

Output from hydroelectric generation will be valued as renewable power at market prices. This study assumes that project output would be classified as incremental hydropower under Washington State's Energy Independence Act because the hydropower component is secondary to the flood reduction and summer flow augmentation purposes.

The Upper Chehalis hydroelectric project output is estimated at 39,952 MWh annually while the South Fork Chehalis project produces 7,401 MWh each year. The energy is valued at average annual price (melded peak and off-peak) at the Mid-Columbia (Mid-C) trading hub. Monthly peak and off-peak energy prices from 2009 (Mid-Columbia) are escalated at according to the escalation rates from the Northwest Power and Conservation Council's Mid-Columbia price forecast from the 6th *Power Plan*. Fifty-year levelized⁴³ costs are approximately \$82/MWh.⁴⁴

Additional value is assigned to energy output to account for the renewable qualities. The renewable energy is valued at \$10/MWh in 2010 and escalated at 2.5 percent (50-year levelized value is \$15/MWh). Tables 25 and 26 show monthly energy and benefits for the Upper Chehalis and South Fork Chehalis hydroelectric projects, respectively.

⁴³ Levelized costs are equal to the present value of a stream of costs (in this case energy) over a period of time, converted to equal annual payments.

⁴⁴ These power costs are conservative. Higher cost resources, renewable portfolio standards, and increasing energy costs in the future will likely lead to higher market prices for electricity over the planning period.

Table 25
Upper Chehalis Energy Value
50-Year Levelized Annual 2010 Dollars

	Output		Price Forecast		Total Benefit	
	On peak	Off peak	On peak	Off peak	Energy	Renewable Credits
	MWh	MWh	\$/MWh	\$/MWh		\$15/MWh
January	4,964	2,482	\$96.04	\$102.77	\$731,852	\$110,671
February	4,486	2,243	\$91.08	\$106.56	\$647,643	\$100,018
March	4,785	2,392	\$72.13	\$81.97	\$541,191	\$106,666
April	818	409	\$53.99	\$51.20	\$65,063	\$18,225
May	874	437	\$54.85	\$48.72	\$69,212	\$19,479
June	841	421	\$44.78	\$32.72	\$51,429	\$18,754
July	850	425	\$80.71	\$76.53	\$101,143	\$18,952
August	823	411	\$92.71	\$86.43	\$111,822	\$18,340
September	770	385	\$89.78	\$73.29	\$97,403	\$17,176
October	781	391	\$104.35	\$105.59	\$122,810	\$17,422
November	1,704	852	\$80.40	\$83.67	\$208,340	\$38,000
December	4,938	2,469	\$126.29	\$134.49	\$955,698	\$110,089
Total	26,635	13,317	\$82.26	\$81.99	\$3,703,607	\$593,793

Table 26
South Fork Chehalis Energy Value
50-Year Levelized Annual 2010 Dollars

	Output		Price Forecast		Total Benefit	
	On peak	Off peak	On peak	Off peak	Energy	Renewable Credits
	MWh	MWh	\$/MWh	\$/MWh		\$15/MWh
January	857	428	\$96.04	\$102.77	\$126,324	\$19,103
February	827	413	\$91.08	\$106.56	\$119,325	\$18,428
March	840	420	\$72.13	\$81.97	\$95,013	\$18,727
April	139	70	\$53.99	\$51.20	\$11,077	\$3,103
May	166	83	\$54.85	\$48.72	\$13,188	\$3,712
June	161	80	\$44.78	\$32.72	\$9,843	\$3,589
July	156	78	\$80.71	\$76.53	\$18,597	\$3,485
August	141	70	\$92.71	\$86.43	\$19,120	\$3,136
September	122	61	\$89.78	\$73.29	\$15,366	\$2,710
October	119	59	\$104.35	\$105.59	\$18,642	\$2,645
November	631	316	\$80.40	\$83.67	\$77,152	\$14,072
December	775	388	\$126.29	\$134.49	\$150,068	\$17,287
Total	4,934	2,467	\$82.26	\$81.99	\$673,715	\$109,994

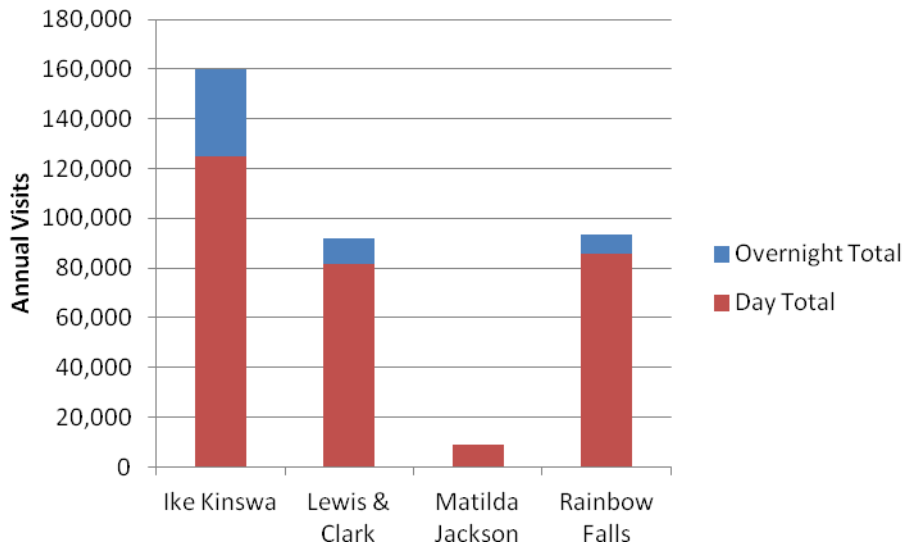
Recreation

Recreational use of the proposed facilities may provide some regional benefit. The information below provides a very high level overview of potential benefits; additional study is needed to better determine the potential benefits.

The number of annual visits to the reservoirs is based on the Corps' REAS model output for a similarly situated reservoir in Washington at Mud Mountain Dam.⁴⁵ The Mud Mountain Dam on the White River, outside of Enumclaw, WA receives 84,702 day-use visitors per year. Mud Mountain reservoir holds 106,000 ac-ft of water and is approximately 5.5 miles long. This reservoir is similar in size to the two projects together. Therefore, the number of annual visits to the proposed reservoirs is estimated at 84,702, with approximately 63,527 and 21,176 annual visits based on weighting the size of the reservoirs for the Upper Chehalis and South Fork Chehalis Reservoirs respectively. Recreation activities at the proposed reservoirs and Mud Mountain Dam are expected to be similar and include hiking, biking, equestrian, swimming, fishing, boating, and picnic. For reference, visits to nearby parks in Lewis County are provided in Figure 13.

⁴⁵ While significant differences exist between the proposed flood control impoundments and Mud Mountain Dam, it does provide the best available analog for an artificial lake in this part of the state. Licenses and permits for the proposed flood control reservoirs can contain conditions that enhance their recreational value.

**Figure 13
State Park Annual Visitation, Lewis County**



Annual visitation for the proposed reservoirs appears consistent with the above data from nearby state parks. For the analysis, the number of total visits to the reservoirs is adjusted to account for site substitution or for visits to the area that occur presently. Possible site substitution might be from Mayfield Lake or Riffe Lake both fed by the Cowlitz River. The number of visits to the proposed reservoirs is reduced by half to account for site-substitution. The resulting number of recreation visits added is likely conservative. Currently, there are no published hiking, biking, or equestrian trails near the structure sites. Therefore, current recreation visits to these areas are expected to be minimal.

The value of a recreation visit is based on the 2005 Bowker study⁴⁶ which values consumer surplus for recreation visits to wilderness areas at \$22.84 per visit (2010 dollars) for the Western United States based on a survey of sites. Table 27 summarizes the recreation visit calculations.

⁴⁶ Bowker, J.M. et al., “The Net Economic Value of Wilderness,” *The Multiple Values of Wilderness* (2005), 169. http://www.srs.fs.usda.gov/pubs/ja/ja_bowker007.pdf

**Table 27
Recreation Visits and Value
2010 Dollars**

Annual Visits, total		84,702	
Site Substitution		50%	
Incremental Annual Visits		42,351	
Value per person per year		\$22.84	
	Share, %	Net Annual Visits	Annual Value
Upper Chehalis	0.75	31,763	\$725,360
South Fork Chehalis	0.25	10,588	\$241,787
Total	1.00	42,351	\$967,146

Costs

Costs to build and maintain the facilities are included to produce net recreation benefits. In the past, annual expenditures on Lewis and Clark State Park have totaled \$287,410.⁴⁷ Construction costs for a boat ramp, parking, and picnic areas are estimated according to the 2002 INEEL study referenced for O&M costs. Table 28 summarizes the construction costs and O&M costs assumed for the recreation facilities.

**Table 28
Recreation Costs
2010 Dollars**

O&M		\$287,410	
Construction, Upper Chehalis		\$5 million	
	Share, %	Annual O&M	Construction
Upper Chehalis	0.75	\$215,558	\$2,692,424
South Fork Chehalis	0.25	\$71,853	\$565,122
Total	1.00	\$287,410	\$3,257,546

The 50-year present value of net benefits due to recreation (includes O&M and construction) is estimated at \$13.8 million and \$4.9 million for the Upper Chehalis and South Fork Chehalis structures respectively.

⁴⁷ Washington Trails Association. *Projected Park Closures*. Accessed September 24, 2010. Available online: <http://www.wta.org/trail-news/magazine/projected-state-park-closures>

National Economic Development Benefit-Cost Analysis

Two benefit-cost analyses were conducted under the National Economic Development (NED) account. The first considers only flood reduction benefits. The second allows for multi-purpose benefits, such as the value of hydropower production. These benefits are shown below. Costs for these two analyses are the same.

Flood Reduction Benefits

Flood reduction benefits include avoided costs such as clean-up, public assistance, relocation, transportation delays, and damages to crops, structure, content, and inventory. Recall that the expected annual benefit is the area between the with-project damage curve and the without-project damage curve as shown in Figure 14.

Figure 14
Example of Reduction in Damage Curve

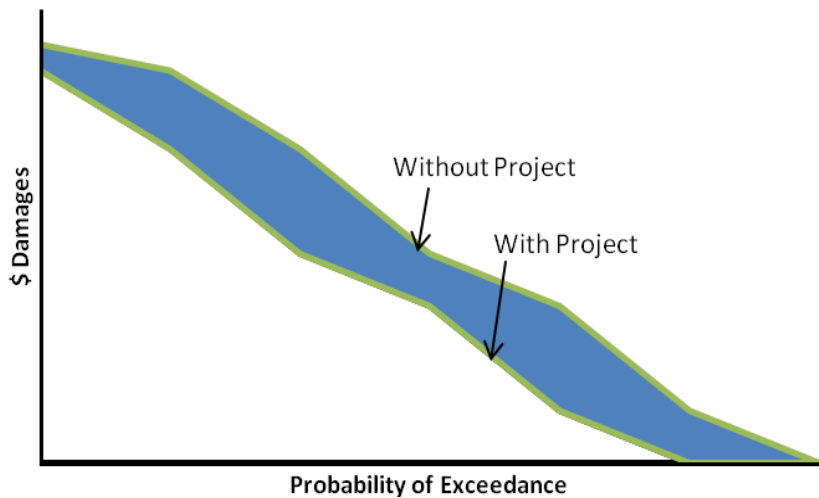


Table 29 shows the annual avoided damages for all of these categories by flood return interval. Using trapezoidal integration, the expected annual benefit is calculated and shown in the last column of Table 29.

Table 29
Avoided Damages

2010 Dollars

	10-Year	50-Year	100-Year	500-Year	Expected Annual Benefit
Upper Chehalis	\$33,273,327	\$121,205,679	\$165,830,126	\$283,573,558	\$9,979,101
South Fork	\$9,799,475	\$32,874,143	\$45,096,879	\$175,242,701	\$3,328,644
Both Projects	\$42,011,750	\$141,380,994	\$188,067,782	\$347,466,743	\$11,820,025

Table 29 shows that total expected annual benefit of \$9,979,101 for the Upper Chehalis project, \$3,328,644 for South Fork project and \$11,820,025 for both projects. The total estimated NED damages in the “with-out project” case are \$443 million for a 100 year flood and \$812 million for a 500 year flood. For comparison, the December 2007 flood caused an estimated \$500 million in damages in Lewis County.⁴⁸

In addition to the avoided damages by flood event listed above, another flood reduction benefit is the avoided cost of raising I-5 as long as certain levees are also modified. The net present value (NPV) of this benefit is approximately \$65 million in 2010 dollars.

Multi-Purpose Project Benefits

The multi-purpose project benefits include the flood reduction benefits listed above, and the added benefits of hydroelectric generation and renewable energy credits. Table 30 shows the expected annual benefit for flood reduction and energy values. Note that potential benefits from flow augmentation cannot be included in this kind of analysis, and are examined in the next section on the Alternative Analysis.

**Table 30
Multi-Purpose Project Benefits
50-Year NPV, 2010 Dollars**

	Flood Reduction Benefits	Energy	RECs	Recreation	Total
Upper Chehalis	\$235,318,195	\$78,358,392	\$12,563,076	\$8,200,289	\$334,439,952
South Fork	\$70,425,166	\$14,253,998	\$2,327,187	\$3,052,615	\$90,058,967
Both Projects	\$274,267,210	\$92,612,390	\$14,890,263	\$5,638,376	\$387,408,239

Table 31 compares the results of the two NED analyses. The costs shown in Table 31 include construction, financing, and operation and maintenance costs.

⁴⁸ Lewis County 2007 Flood Disaster Recovery Strategy. Published April 2009 by the Cowlitz-Wahkiakum Council Governments.

Table 31
NED Benefit-Cost Ratios, 50-Year Period
2010 Dollars

	Benefit	Cost	Benefit-Cost Ratio
Flood Reduction Project			
Upper Chehalis	\$235,318,195	\$206,766,205	1.14
South Fork	\$70,425,166	\$105,352,985	0.67
Both Projects	\$274,267,210	\$312,119,190	0.88
Multi-Purpose Project			
Upper Chehalis	\$334,439,952	\$296,479,010	1.13
South Fork	\$90,058,967	\$162,338,251	0.55
Both Projects	\$387,408,239	\$458,817,261	0.84

Alternative Analysis

This section of the report monetizes possible environmental benefits of the projects. These benefits are included in both the Alternative and Regional analyses. The scope of work included analysis of environmental benefits and costs to be quantified for benefit-cost analysis. This analysis is outside the Corps' *Principles & Guidelines* methodology, but has been used elsewhere in the United States following Hurricane Katrina.

Background

The precedent for quantifying the project ecosystem benefits comes from the current work to restore the Mississippi River Delta. The Louisiana State legislature approved the use of multi-criteria methodology where the preferred alternative is selected based on all accounts (benefit-cost analysis, wetlands restoration, and public safety). As a result of the Louisiana legislature's decision, Earth Economics⁴⁹ has examined the dollar value of storm protection, carbon sequestration, and other ecosystem benefits provided by the wetlands of the Mississippi Delta. The valuation of these ecosystem and public safety benefits may show that protection and restoration of wetlands in conjunction with smaller levees is more economically efficient in providing hurricane protection than levee construction alone. These principles provide a construct for monetizing ecosystem benefits that may produce a more balanced analysis of the proposed Chehalis River projects.

Earth Economics has conducted a high-level review of the methodologies used in this report to monetize environmental benefits from the proposed structures. While the methodology is not approved by Earth Economics, Dave Batker provided general suggestions to update the analysis and offered that while the analysis is rough, it is not unreasonable. It was suggested that for a more accurate study, the GIS model developed for the Chehalis basin should be used to determine the exact benefits from reduced flooding. Based on the review by Earth Economics, EESC decided that using the lowest value estimate from Earth Economics study would be appropriate for a conservative analysis.

Net Environmental Benefits

Reservoir Value

The proposed structures would result in several changes to the natural environment in the Chehalis watershed. Most notably, a portion of the river, riparian habitat, and surrounding terrestrial territory would be converted to reservoir habitat. These changes in habitat are valued using an ecosystem analysis by Earth Economics⁵⁰ and result in a range of values of \$88,495 to

⁴⁹ Earth Economics. *Flood Protection and Ecosystem Services in the Chehalis River Basin*. May 2010. Tacoma, WA.

⁵⁰ Earth Economics has provided a high-level review of the environmental analysis and has provided suggestions for improvement. Due to the high-level nature of the review, Earth Economics does not endorse or approve the results of the analysis.

\$25,055,178. The discussion below provides the basis for these figures. Given the uncertainties associated with these figures and the need for further study, the lowest figure of \$88,495 was used in the benefit/cost analysis.

Upper Chehalis Site

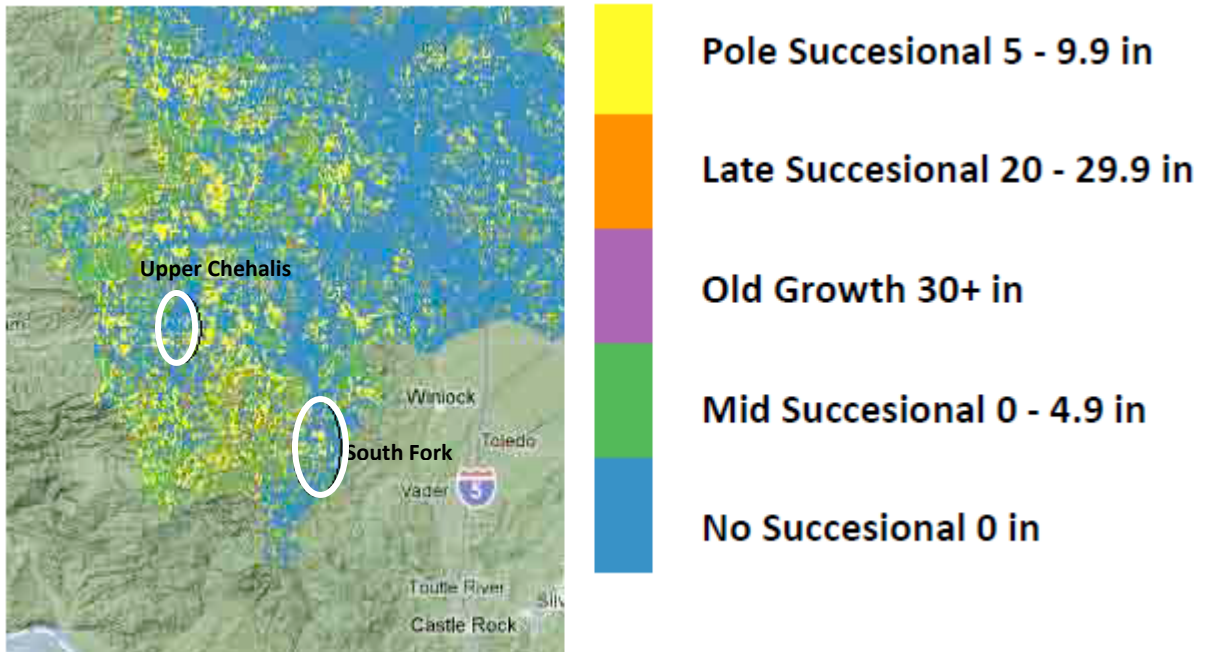
Construction of a potential water retention facility at the Upper Chehalis River site would result in a reservoir with about 940 acres of surface area when filled to the maximum operating level for flow augmentation and hydropower production. The riparian habitat of the Upper Chehalis in the area would be covered by the new reservoir. About 9.6 river miles of the Upper Chehalis would be covered by the full reservoir. Assuming an average river bed width is 40 feet in this reach, about 46.5 acres of river habitat would be inundated and converted to still water (reservoir) habitat. About 940 acres of terrestrial habitat, mostly forested lands, would be lost due to the inundation, but it would be replaced with 940 acres of lake/reservoir habitat.

South Fork Chehalis Site

Construction of the proposed water retention facility at the South Fork of the Chehalis River would result in a reservoir with about 300 acres of surface area at the maximum operating level for flow augmentation and hydropower production. The existing riparian habitat of the South Fork in this area would be covered by the new reservoir. About 4.6 river miles of the South Fork would also be covered. Assuming an average river bed width is 30 feet in this reach about 16.7 acres of river habitat would be inundated and converted to still water (reservoir) habitat. About 300 acres of terrestrial habitat, mostly forested lands, would be lost due to the inundation, but it would be replaced with 300 acres of lake/reservoir habitat.

Figure 15 illustrates roughly where the two reservoirs are proposed to be located and the types of forest that characterize those areas. The figure shows that the reservoirs would replace “no successional” to “mid-successional” forested areas. As an average value for the lost terrestrial habitat, the value for pole forest from the Earth Economics study is used. According to the Earth Economics Study pole forest habitat is valued in the range of \$12 to \$636 per acre.

Figure 15
Reservoir Location and Forest Successional Stages⁵¹



⁵¹ Earth Economics. *Flood Protection and Ecosystem Services in the Chehalis River Basin*. May 2010. Tacoma, WA.

Table 32 shows a range of values for several types of ecosystems within the Chehalis River Basin.

Table 32					
Ecosystem Values for the Chehalis River Basin					
Source: Earth Economics, 2010					
<i>Cover Type</i>	<i>Acres</i>	<i>Low</i>	<i>High</i>	<i>Low</i>	<i>High</i>
Early Forest	481,420	\$7.52	\$881.45	\$3,620,278.40	\$424,347,659.00
Pole Forest	232,275	\$7.52	\$881.45	\$1,746,708.00	\$204,738,798.75
Mid Forest	289,667	\$73.42	\$1,093.26	\$21,265,902.81	\$316,679,896.09
Late/Old Forest	78,243	\$395.77	\$2,420.48	\$30,966,232.11	\$189,385,616.64
Riparian forest pole	43,068	\$35.49	\$12,567.43	\$1,528,267.98	\$541,254,075.24
Riparian Forest mid to late	38,020	\$3,468.97	\$25,365.19	\$131,890,239.40	\$964,384,523.80
Riparian Shrub	4,176	\$35.49	\$12,567.43	\$148,185.36	\$52,481,587.68
Fresh Wetland	104,395	\$6,676.61	\$59,914.27	\$697,004,700.95	\$6,254,750,216.65
River/Lakes	35,931	\$77.71	\$22,013.28	\$2,792,198.01	\$790,959,163.68
Shrub/Scrub	177,302	\$7.52	\$881.45	\$1,333,311.04	\$156,282,847.90
Grassland/herb	87,479	\$97.36	\$97.36	\$8,516,955.44	\$8,516,955.44
Agriculture	12,785	\$29.90	\$39.60	\$382,271.50	\$506,286.00
Pasture	73,153	\$6.25	\$6.25	\$457,206.25	\$457,206.25
Urban green space	78,046	\$1,293.84	\$4,743.10	\$100,979,036.64	\$370,179,982.60
Beach	2,188	\$22,353.32	\$81,528.01	\$48,909,064.16	\$178,383,285.88
Salt Marsh	4,876	\$358.74	\$114,739.48	\$1,749,216.24	\$559,469,704.48
Eel grass beds	36,419	\$5,507.00	\$15,421.00	\$200,559,433.00	\$561,617,399.00
Estuary Waters	21,010	\$18.62	\$1,868.51	\$391,206.20	\$39,257,395.10
Marine Waters	40,102	\$259.34	\$772.68	\$10,400,052.68	\$30,986,013.36
Snow and Ice	23	\$0.00	\$0.00	\$0.00	\$0.00
Barren and developed land	73,816	\$0.00	\$0.00	\$0.00	\$0.00
Totals	1,914,394			\$1,263,686,498.12	\$11,644,638,613.54

According to *Flood Protection and Ecosystem Services in the Chehalis River Basin (2010)*, ecosystem services provided by lake and river habitat are valued the same.^{52, 53} Ecosystem benefit provided by pole forest or early forest habitat is generally of less value than lake or river

⁵² Evaluating the merit of this equal valuation is beyond the scope of this study. Further research and analysis will be needed on this subject, especially in view of the different habitat and life history requirements of fish species present in the river system. Additional information provided by the Anchor QEA Fisheries Study will be helpful for future analysis in this area.

⁵³ Ecosystem services as computed here do not comprehensively include recreational benefits.

habitat. Since lake and river habitat are assigned the same value, the lost river habitat value is cancelled by the gained lake habitat value.

Table 33 demonstrates the calculation used to value the change from terrestrial and riparian habitat to reservoir habitat. The incremental benefit of reservoir habitat was applied to the lost terrestrial acres (pole or early forest).

Table 33 Annual Reservoir Value in 2010 Dollars				
		Low	High	
Pole Forest, \$/Acre		\$7.52	\$881.45	
Reservoir (River/Lakes), \$/Acre ⁵⁴		\$77.71	\$22,013.28	
<hr/>				
Added benefit of Lake, \$/Acre		\$66.00	\$21,131.83	
Riparian, \$/Acre		\$35.49	\$12,567.43	
Reservoir (River/Lakes), \$/Acre		\$77.71	\$22,013.28	
<hr/>				
Added benefit of Lake, \$/Acre		\$42.22	\$9,445.85	
	Added Reservoir Acres	Lost River Acres	Lost Terrestrial/Pole Forest Acres	Lost Riparian Pole Forest Acres
South Fork Chehalis Reservoir	300	16.7	283.3	21.9
Upper Chehalis Reservoir	940	46.5	893.5	103.0
<hr/>				
Added Benefit		Low	High	
South Fork Chehalis Reservoir		\$21,059	\$6,062,146	
Upper Chehalis Reservoir		\$67,436	\$18,993,033	
<hr/>				
Total Benefit		\$88,495	\$25,055,178	

Water Quality

The flood retention structures would change the characteristics of the river downstream of the impoundment and could help improve summertime water quality parameters. However, the potential benefits and impacts of water storage, especially on temperature during the summer months, needs additional study. Although some considerations are described below, no potential monetary benefit is included in this analysis due to the need for additional information.

⁵⁴ Reservoir ecosystem value and lake/river habitat value are not the same due to several differences. Reservoirs generally increase greenhouse gas (GHG) emissions and reduce carbon sequestration when forests are inundated; however, if forest harvest periods are lengthened as a result of the reservoir, GHG emissions may be lower.

Upper Chehalis Water Quality

Water quality is of particular concern in the Upper Chehalis during the summer time. Currently, estimated average stream flow at the Upper Chehalis site is 50 cfs from June through September, or approximately 14 percent of the average annual daily flow of 348 cfs. These low summer flows and high temperatures in the summer cause fish kills in some years. For example, extensive fish kills occurred in the Chehalis and Newaukum Rivers during periods that coincided with high temperatures⁵⁵ in the summer of 2009. The Washington Department of Ecology has documented high water temperatures at Doty nearly every summer since it began making measurements it is not uncommon that the lethal temperatures are recorded for at least one week per year. During these periods, fisheries biologists confirmed that high temperatures resulted in fish mortality of adult salmon and other fish species below Pe Ell on the mainstem. In August 2009, fisheries biologists conducted snorkeling surveys to determine if juvenile salmonids were present at six sites on the Chehalis River from upstream of Pe Ell to 15 miles downstream. The survey found no adult or juvenile species present downstream of Pe Ell. Recorded temperatures for these waters ranged from 68 degrees at Pe Ell to 79 degrees three miles downstream from Pe Ell. Studies have shown that adult salmon and steelhead begin to die at water temperatures in the 69.8 to 71.6 degree range (Fahrenheit).⁵⁶ The biologists concluded that high river temperatures in the Chehalis River during hot summer months can be lethal to salmon, steelhead, and several other species.

Upstream storage could help increase summertime flows. Under the multi-purpose operation design developed for this Phase IIB study, average daily flow at the Upper Chehalis site would be 140 cfs in the summer months in an average water year. These augmented flows are nearly three times the estimated average natural flow (40 cfs). Based on nearly triple the natural flow, and potentially cooler than natural water temperatures, it can be inferred that fish kill incidents might be greatly reduced for up to several miles downstream of the water retention sites. However, additional study on this question is needed and the benefits of any potential improvements are not included in this study.⁵⁷

South Fork Chehalis Water Quality

Similar to the mainstem Chehalis, the South Fork Chehalis also has low summertime flows. During an average water year, average daily flow from June through September is estimated at 16.4 cfs, or 14.4% of average annual daily flow using the Doty gage approximation. Augmented flows during the June through September timeframe could be 40 cfs or an average of 240 percent of natural flow in an average year. Detailed flow studies would be required to determine the true

⁵⁵ Kohn, Rittmueller, and Warnock 2010

⁵⁶ Kohn, Rittmueller, and Warnock 2010

⁵⁷ WDOE and Washington Department of Fish and Wildlife (WDFW) documented the relationship between Chinook habitat availability and streamflow. Chinook spawning habitat decreases rapidly as streamflows decrease below 160 – 170 cfs in the Upper Chehalis River; rearing habitat experiences a similar decrease when streamflows drop below 70 cfs. Limiting habitat factors for Chum are similar. (EES Consulting, Inc. November 10, 2009).

amount of summer flow augmentation. In addition, specific environmental studies are required to evaluate the improved water quality/habitat and associated value.

Additional Benefits Needing Further Study

The proposed multi-purpose flood retention structures may provide several other benefits since they have the capacity to store high winter flows and then to release those flows during the summer months when natural flows on the Chehalis River are typically low. Summer flow augmentation may provide significant benefit to wildlife habitat, and could make additional water available for agricultural use or rural and municipal consumption, providing significant economic and social benefits to local farmers and communities. In addition, greater summer flows may recharge ground water supplies. Given improved water quality and improved fish habitat, greater production in salmon and steelhead species could promote economic development. However, the benefit of such summer flow augmentation have not been studied, nor have they been incorporated into the Phase IIB benefit-cost analysis.

Wetland Benefits

Augmented summer flows may provide opportunities for wetland creation. Wetlands are known to have the ability to mitigate flooding events near urban areas.⁵⁸ Wetlands near the Chehalis and Centralia may help to regulate water flows such that flooding is reduced further than indicated in this study.

Direct impacts, positive or negative, from the projects to existing freshwater wetlands were not evaluated specifically since that is beyond the scope of a Phase IIB Feasibility Study and the Corps' guidelines for such studies; however, the value of additional wetlands was estimated based on work completed by Earth Economics. It is not known what type of wetlands might be created (off channel slough or other) nor is it assumed that existing wetlands are inundated. Due to low summer flows, the acres of existing wetlands that might be inundated are expected to be insignificant. Wetland benefits were calculated assuming that added wetland areas are downstream from any water quality benefits. This analysis assumes that wetlands are relatively inexpensive to create and that landowners are compensated at market prices for land converted to wetland habitat. Based on flood-depth maps, it was estimated that the summer augmentation flows (90 cfs of additional water) from the Upper Chehalis site could be used to convert 179 acres of pasture⁵⁹ to wetland habitat. Landowners are compensated \$2,000 per acre⁶⁰ for a total cost of \$396,000. Using fresh water wetland values developed by Earth Economics, Table 34 summarizes a range of values for wetland creation from the Upper Chehalis multi-purpose project site.

⁵⁸ Batker, David et al. June 2010. *Gaining Ground. Wetlands, Hurricanes, and the Economy: The Value of Restoring the Mississippi River Delta*. Earth Economics, Tacoma, WA.

⁵⁹ Pastureland values are used in the analysis, however, it is likely that created wetlands might be located on land that is not currently used and therefore has a value of \$0/acre for ecosystem services.

⁶⁰ Value based on 2010 taxable land value for representative property near the South Fork Chehalis River.

Table 34
Upper Chehalis Wetland Added Annual Benefit
2010 Dollars

Added Wetland Acres	179		
Landowner Compensation, \$/acre	\$2,000		
Total Compensation	\$357,025		
	Wetland Value	Pasture Value	Added Value
	\$/Acre	\$/Acre	\$/Acre
Low	\$6,677	\$6.25	\$6,670
Average	\$33,295	\$6.25	\$33,289
High	\$59,914	\$6.25	\$59,908
	Wetland Annual		Wetland Annual
	Added Value		Net Added Value
Low	\$1,190,742		\$833,717
Average	\$5,942,533		\$5,585,508
High	\$10,694,324		\$10,337,299

Based on a similar analysis, the augmented summer flows from the South Fork multi-purpose project site could support over 51 acres of new wetland by providing reliable water supplies year-round. Estimated augmented flow is 40 cfs. Estimated natural average annual daily flow during summer months is 14 cfs. The resulting additional flow is therefore 26 cfs. Table 35 summarizes wetland benefits from the South Fork site.

Table 35
South Fork Chehalis Wetland Added Annual Benefit

Added Wetland Acres	52		
Landowner Compensation, \$/acre	\$2,000		
Total Compensation	\$103,140		
	Wetland Value	Pasture Value	Added Value
	\$/Acre	\$/Acre	\$/Acre
Low	\$6,677	\$6.25	\$6,670
Average	\$33,295	\$6.25	\$33,289
High	\$59,914	\$6.25	\$59,908
	Wetland Annual		Wetland Annual
	Added Value		Net Added Value
Low	\$343,992		\$240,852
Average	\$1,716,732		\$1,613,591
High	\$3,089,471		\$2,986,331

These wetland benefits are calculated as a placeholder until targeted, independent studies can review the costs and benefits of introducing additional wetland areas in the Chehalis River Basin.

Fisheries Analysis

Fisheries benefits and costs are being studied by separate contract by Anchor QEA for the Flood Authority. The results of that effort will not be known until mid-2011, and are not included in this report.

Alternative Benefit-Cost Analysis

Summarizing the information above, the Alternative benefit-cost analysis includes monetized net environmental benefits. The low estimate of net environmental benefits is shown in Table 36.

Table 36			
Alternative Benefit-Cost Analysis Project Benefits			
Net Present Value, 2010 Dollars			
	Multi-Purpose Project Benefits	Net Environmental Benefit	Total
Upper Chehalis	\$334,439,952	\$27,435,614	\$361,875,566
South Fork	\$90,058,967	\$8,889,187	\$98,948,153
Both Projects	\$387,408,239	\$36,324,801	\$423,733,040

Table 37 shows the results of benefit-costs ratios for the Alternative Analysis.

Table 37			
Alternative Benefit-Cost Ratios, 50-Year Period			
NPV 2010 Dollars			
	Benefit	Cost	Benefit-Cost Ratio
Upper Chehalis	\$361,875,566	\$296,479,010	1.22
South Fork	\$98,948,153	\$162,338,251	0.61
Both Projects	\$423,733,040	\$458,817,261	0.92

Regional Analysis

This section describes the benefits and costs of the Chehalis River projects from a regional perspective. Benefits added to the Alternative Analysis reflecting regional concerns include property value increases, avoided business income losses, and intrinsic value of improved water quality on the mainstem of the Chehalis River (in summer months).

Property Values

Increases in property values due to flood reduction were included in the 2009 cost-benefit analysis (Phase I). These values were estimated using flood insurance premiums. The property values from the 2009 report were updated and included as regional benefits in this analysis.

Flood insurance premium information was also used to calculate potential increases in property value due to decreased flooding for some properties once a project is built. In general, flood insurance is required for properties located within the 100-year flood plain. Once one or more of the proposed projects is built, some properties that previously flooded would remain dry. EESC used an estimate for the average home and commercial property value, national flood insurance premiums, and output from the HAZUS model to estimate the avoided insurance premiums resulting from the projects. HAZUS estimates the number of buildings that are flooded in a flood event. For the property value analysis, it is assumed that there will be benefit only to homes that are no longer flooded during a 100-year event or less in the with-project case. EESC also assumed that homes no longer flooded during a 100-year event have more value than homes no longer flooded at a lesser event, since these buildings would be essentially removed from the 100-year flood plain. Table 38 shows the buildings that are no longer flooded once the projects are in place.

**Table 38
Now-Dry Buildings in With-Project Cases
From HAZUS Output**

	10-Year	50-Year	100-Year	500-Year
Commercial Buildings No Longer Flooded				
Upper Chehalis	1	8	5	12
South Fork Chehalis	0	4	1	4
Both	1	8	8	14
Residential Buildings No Longer Flooded				
Upper Chehalis	37	69	184	755
South Fork Chehalis	16	31	97	108
Both	43	97	213	941

Table 39 shows the annual flood insurance premiums from National Flood Insurance Program website. Residential coverage is for buildings with an average value of \$100,000. Commercial coverage is for buildings with an average value of \$200,000.⁶¹

Table 39
Annual Flood Insurance Premiums
2010 Dollars

	Standard Risk		High Risk	
	Residential	Commercial	Residential	Commercial
Annual Premium	\$598	\$1,394	\$795	\$1,851
30-Year NPV	\$10,185	\$23,743	\$13,541	\$31,527

It was assumed that a building no longer flooded during the 10-year and 50-year events would pay lower premiums. Buildings no long flooded in the 100-year event would not purchase insurance and would save high-risk insurance premiums. Buildings no longer flooded at the 500-year event did not receive increases in value. Annual premiums were applied over a 30-year period. The net present value (NPV) of the payment stream was calculated so that a home no longer flooded by the 100-year event (high risk) avoids approximately \$13,541 in cost over the life of a 30-year mortgage. Therefore, homes no longer flooded under a 100-year scenario increase in value by \$13,541. This was a one-time increase applied in the first year of the analysis. Table 40 shows the total property value increase as estimated in this section.

Table 40
Total Property Value Increase
2010 Dollars

Buildings No Longer Flooded at:	10-Year	50-Year	100-Year	500-Year
Upper Chehalis Project Benefit	\$131,934	\$169,645	\$1,714,841	\$0
South Fork Chehalis Project Benefit	\$53,687	\$81,467	\$925,227	\$0
Benefit from Both Projects	\$152,067	\$243,464	\$1,822,963	\$0

Direct and Indirect Economic Losses

The HAZUS model calculates economic losses to local businesses and businesses affected by delayed supply from the flooded area. The HAZUS model uses the IMPLAN input-output model to estimate indirect economic losses. Direct and indirect economic losses in Table 41 are included only as a regional scenario in the analysis. The national economic development account calculated per the *Principles & Guidelines* does not include regional business losses. The losses below include income and wages as well as rental income losses.

⁶¹ These average structure values are conservative.

Table 41
HAZUS Output: Regional Economic Loss
2010 Dollars

	10-Year	50-Year	100-Year	500-Year
Base	\$793,718	\$1,308,562	\$1,737,599	\$2,966,947
With Upper Chehalis Structure	\$611,377	\$1,018,962	\$1,201,303	\$1,880,814
Upper Chehalis Project Benefit	\$182,341	\$289,600	\$536,296	\$1,086,133
With South Fork Chehalis Structure	\$729,362	\$1,222,755	\$1,512,355	\$2,735,109
South Fork Chehalis Project Benefit	\$64,356	\$85,807	\$225,244	\$231,838
With Both Projects	\$568,474	\$933,155	\$1,094,044	\$1,683,969
Benefit from Both Projects	\$225,244	\$375,407	\$643,555	\$1,282,978

Intrinsic Value

The 2009 Phase I cost-benefit analysis included an estimate of the intrinsic value of water quality in the Chehalis River Basin. Intrinsic values encompass a wide array of water uses that are indirectly related to the water source. These may include ecological value, preservation benefits and option or bequest values (Koteen et. al 1998). There is an inherent value for the existence of the river and the quality of the river. By simply existing, the river provides value to nearby ecosystems, residents, and future generations, without direct use. Intrinsic values (non-recreation values) are estimated on a per household basis for the following towns: Centralia, Chehalis, Napavine, and Pe Ell. These estimates account for existence and bequest values, and exclude any values associated with the fishing industry.

Valuation studies are used to determine the intrinsic value per household for nearby residents. The Corps does not include intrinsic values in their evaluation of water resources; therefore, intrinsic values were included only in the Alternate Analysis. The value of augmented summer flows was estimated using values from an ecosystem study (Earth Economics 2010). The value of augmented flows does not include intrinsic valuations of the Chehalis River water quality. This section, therefore, adds subjective estimates for the intrinsic value gained from better water quality during summer months in the Chehalis River. Additional research will be needed to provide more authoritative, quantitative estimates of these parameters.

Table 42 shows the number of households affected by the projects using population data from the Washington State Office of Financial Management (OFM). 2010 population projections and an average household size of 2.4 (people) is used to estimate the number of households. Table 42 also shows that households in different towns are affected by only the neighboring or upstream project.

**Table 42
Households Affected**

	2010 Households	Affected by Project:
Napavine	715	South Fork Only
Pe Ell	279	Upper Chehalis Only
Centralia	6,488	Both Projects
Chehalis	2,994	Both Projects

Using an intrinsic value of \$129.44/household/year (2010 dollars),⁶² intrinsic value for the low-flow months (June-September) is calculated at \$43.15/household/year. This value is escalated by inflation and applied to the number of households (escalated by historic growth rates) to produce the intrinsic value for water quality improvements in the basin. Table 43 summarizes the results of the analysis for the first year of the study period.

**Table 43
Intrinsic Value, Year 1
2010 Dollars**

	Households	\$/Year	Population Growth
Upper Chehalis Project Benefit	7,864	\$339,307	0.37%
South Fork Chehalis Project Benefit	715	\$30,831	3.26%
Benefit from Both Projects	10,475	\$451,954	0.53%

Regional Benefit-Cost Analysis

For the regional analysis, expected benefit due to avoided business losses, intrinsic value of better water quality in summer, and property values were added to the Alternative Analysis benefits. Table 44 shows the 50 year NPV of the regional benefits by component.

⁶² The 2009 study used \$100/household/year in 1998 dollars according to Koteen et al.

Table 44
Regional Benefits
50-Year NPV, 2010 Dollars

	Alternative Analysis Benefit	Avoided Business Losses, Expected	Intrinsic Values	Property Value Increase	Total
Upper Chehalis	\$361,875,566	\$670,033	\$7,645,261	\$2,077,114	\$372,267,974
South Fork	\$98,948,153	\$208,480	\$1,235,904	\$1,092,298	\$101,484,835
Both Projects	\$423,733,040	\$833,449	\$10,479,226	\$2,285,271	\$437,330,986

Table 45 shows the results of the Regional Analysis.

Table 45
Regional Benefit-Cost Ratios
50-Year Period, 2010 Dollars

	Benefit	Cost	Benefit-Cost Ratio
Upper Chehalis	\$372,267,974	\$296,479,010	1.26
South Fork	\$101,484,835	\$162,338,251	0.63
Both Projects	\$437,330,986	\$458,817,261	0.95

Benefit-Cost Analysis

Three benefit-cost ratios are provided for each proposed water retention structure option. First, the National Benefit-Cost Analysis is provided. These benefit-cost ratios are developed using the *Principles & Guidelines* methodology used by the Corps and are consistent with the national Water Resources Development Act. Second, an Alternative Benefit-Cost Analysis added monetized environmental benefits to each project. Finally, regional benefits and costs were added for a Regional Benefit-Cost Analysis. The Regional Analysis also included limited monetized environmental benefits.

Summary

Table 46 summarizes the benefits and costs in NPV over a 50-year planning period. The benefit-cost ratio is above one for all scenarios on the Upper Chehalis, below one for the South Fork, and below one if both projects are built.

Table 46
Benefit-Cost Ratios, 50-Year Period
2010 Dollars

	Benefit	Cost	Benefit-Cost Ratio
Upper Chehalis			
Flood Reduction (NED)	\$235,318,195	\$206,766,205	1.14
Multi-Purpose (NED)	\$334,439,952	\$296,479,010	1.13
Alternative	\$361,875,566	\$296,479,010	1.22
Regional	\$372,267,974	\$296,479,010	1.26
South Fork			
Flood Reduction (NED)	\$70,425,166	\$105,352,985	0.67
Multi-Purpose (NED)	\$90,058,967	\$162,338,251	0.55
Alternative	\$98,948,153	\$162,338,251	0.61
Regional	\$101,484,835	\$162,338,251	0.63
Both Projects			
Flood Reduction (NED)	\$274,267,210	\$312,119,190	0.88
Multi-Purpose (NED)	\$387,408,239	\$458,817,261	0.84
Alternative	\$423,733,040	\$458,817,261	0.92
Regional	\$437,330,986	\$458,817,261	0.95

Table 47 shows a range of the benefit-cost results using the high, most likely, and low assumptions presented in Table 3 in the cost section. The benefit-cost ratios for the Upper Chehalis project are generally above one in the different scenarios. The South Fork benefit-cost ratios are never above one. When considered together, both projects may be cost effective depending on the base cost assumptions.

Table 47
Benefit-Cost Ratios with Low, Base, and High Construction Costs
50-Year NPV Analysis

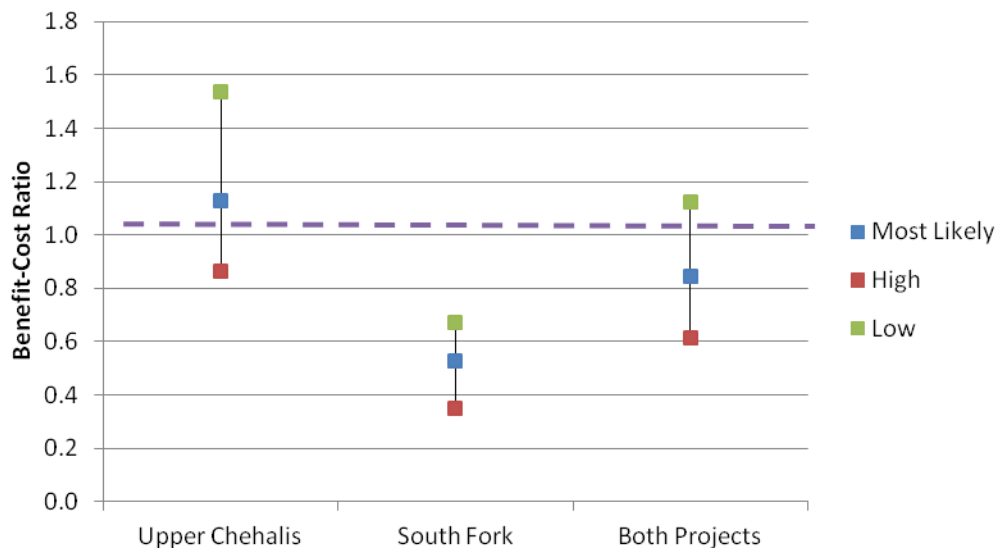
	Low	Most Likley	High
Upper Chehalis			
Flood Reduction (NED)	1.52	1.14	0.83
Multi-Purpose (NED)	1.54	1.13	0.87
Alternative	1.66	1.22	0.94
Regional	1.71	1.26	0.96
South Fork			
Flood Reduction (NED)	0.84	0.67	0.41
Multi-Purpose (NED)	0.71	0.55	0.37
Alternative	0.78	0.61	0.41
Regional	0.80	0.63	0.42
Both Projects			
Flood Reduction (NED)	1.15	0.88	0.61
Multi-Purpose (NED)	1.12	0.84	0.61
Alternative	1.23	0.92	0.67
Regional	1.27	0.95	0.69

Conclusions and Recommendations

Summary

Under the Corps' methodology outlined in the *Principles & Guidelines* documents, the benefit-cost analysis indicates that the Upper Chehalis project is likely cost effective as either a single purpose or multi-purpose facility. The South Fork Chehalis benefit-cost ratios are not favorable. Figure 16 illustrates the range of benefit-cost ratios calculated given different cost assumptions. In most cost scenarios, the Upper Chehalis project is a cost-effective flood reduction project.

Figure 16
Flood Reduction Only Project Benefit-Cost Ratio with Low, Most Likely, and High Construction Costs



Further Review and Refinement

The Phase IIB economic analysis included several benefits and costs according to Corps methodology or precedents set in other parts of the country. These costs and benefits were analyzed within the scope of the study; however, additional costs and benefits might be attributed to the projects if further study is funded. Some of the potential studies and updates that could be pursued are described below.

- Additional study is required to determine effects on fish, habitat, and mitigation requirements, a portion of which is being studied under separate contract for the Flood Authority.
- Additional study on the effect of stream augmentation is required to evaluate temperature changes to fish and wildlife habitat during low flow seasons.

- Additional study of stream augmentation during low flow seasons is needed to evaluate dissolved oxygen, turbidity, sediment transport, and hydrologic impacts on fish habitat and behavior, using a dynamic, vertically stratified model.
- The hydrology model needs to be updated to include Grays Harbor County and Thurston County impacts.
- Additional work is needed to calculate miscellaneous agriculture damages. For example, the 2007 flood event crippled many farms so that they were unable to plant the subsequent season. In addition, damage to dairy inventories, animals, and farming equipment are not accounted for in the analysis. Survey work could be done to determine the exposure to damage from flooding.
- Additional work is needed to conduct a wildlife impact study using a methodology such as the Habitat Evaluation Procedures.
- Additional study is needed on potential benefits to wetlands. For example, the Flood Authority or other stakeholders could hire a consultant to evaluate the impact, if any, of created wetlands on flooding. The value of wetlands is estimated according to existing studies; however, the study would benefit from specific information regarding wetland placement for maximum flood reduction capacities using GIS modeling already developed for the basin.
- Update the HAZUS model with 2010 Census Block Data. When available, using current census data will ensure that economic development since 2000 is accounted for in the damage assessment.
- Refining the Regional Analysis in the future to include replacement value of structures where this report includes depreciated values. Using replacement value would increase the cost-effectiveness of all projects.
- Additional work to incorporate Corps updated benefits information from the Twin Cities Project once it is available.

It is important to note that this study represents one piece of a much larger set of studies should major water retentions facilities be pursued. These kinds of projects involve significant work, time and resources to study, permit, and implement.

References

Batker, David et al. June 2010. *Gaining Ground Wetlands, Hurricanes, and the Economy: The Value of Restoring the Mississippi River Delta*. Earth Economics. Tacoma, WA.

Batker, David, Briana Lovell and Maya Kocian. *Flood Protection and Ecosystem Services in the Chehalis River Basin*. Earth Economics. Tacoma, WA.

Beecher, Hal. Comments from the Washington Departments of Ecology and Fish and Wildlife on Chehalis River Retention Structures Scoping Document and Proposed Studies. Enclosed in the Department of Ecology letter to the Flood Authority Subcommittee. January 7, 2009.

Bowker, J.M. et al., “The Net Economic Value of Wilderness,” *The Multiple Values of Wilderness* (2005), 169. http://www.srs.fs.usda.gov/pubs/ja/ja_bowker007.pdf

Cowlitz-Lewis Economic Development District. April 2009. *Lewis County 2007 Flood Disaster Recovery Strategy*. Kelso, WA.

EES Consulting, Inc. November 2003. *Cowlitz Falls Project Independent Review*. Completed for Lewis Country PUD. Kirkland, WA.

EES Consulting, Inc. February 18, 2009. *Chehalis River Water Retention Facilities Potential Study*. Kirkland, WA.

EES Consulting, Inc. October 26, 2009. *Lewis County PUD #1 Conservation Potential Assessment, Final Report*. Kirkland, WA.

EES Consulting, Inc. November 10, 2009. *Chehalis River Water Retention Structures Scoping Document and Proposed Studies*. Prepared for: Chehalis River Fish and Aquatics Work Group, Chehalis Basin Flood Authority. Revised Working Draft. Bellingham, WA.

Engineers, U. A., June 2003. *Centralia Flood Damage Reduction Project Chehalis River, Washington. Final General Reevaluation Report*. Appendix D: Economics. Department of the Army. Washington D.C.: U.S. Army Corps of Engineers.

Engineers, U. A., April 22, 2000. *Planning Guidance Notebook*. Department of the Army. Washington D.C.: U.S. Army Corps of Engineers.

Hall, Douglas, G. et al. June 2003. *Estimation of Economic Parameters of U.S. Hydropower Resources*. Idaho National Engineering and Environmental Laboratory, Bechtel BWXT Idaho, LLC.

Kohn, Mike, Pete Rittmueller, and Cory Warnock. 2010. *Elevated River Temperatures Result in Salmon Kills in the Chehalis River Basin Summary of Fisheries Investigation July 30-August 27, 2009*. EES Consulting, Inc. Bellingham, WA.

National Oceanic and Atmospheric Association. *Historical Crests for Chehalis River near Doty*. Available online: <http://water.weather.gov/ahps2/crests.php?wfo=sew&gage=dotw1>.

Olson, Patricia. *Comments from WSDOT, Ecology, and Fish and Wildlife, Chehalis River Flood Water Retention Project Phase IIB Feasibility Study (Nov. 10, 2010)*.

Schreiber, Dan. November 26, 2008. *The Flood: One Year On*. The Chronicle Online.

State of Washington Office of Financial Management. *2007 County Projections by Single Year after 2010*. Medium. Accessed May 17, 2010. Available online at: <http://www.ofm.wa.gov/pop/gma/projections07.asp>

Scawthorn, Charles et al. May 2006. *HAZUS-MH Loss Estimation Methodology. I. Overview and Flood Hazard Characterization*. Natural Hazards Review, American Society of Civil Engineers. Reston, VA.

Scawthorn, Charles et al. May 2006. *HAZUS-MH Loss Estimation Methodology. II. Damage and Loss Assessment*. Natural Hazards Review, American Society of Civil Engineers. Reston, VA.

Shannon & Wilson. October 28, 2009. *Reconnaissance-Level Geotechnical Report Proposed Chehalis River and South Fork Dam Sites*. Seattle, WA.

Shannon & Wilson. October 27, 2009. *Geologic Reconnaissance Study Proposed Chehalis River and South Fork Dam Sites*. Seattle, WA.

USGS. Gage 12020000. Real-Time Water Data for the Nation. Available at: <http://waterdata.usgs.gov/nwis/uv?>

USGS Water-Data Report 2009. 12020000 Chehalis River Near Doty, WA. Available at: <http://wdr.water.usgs.gov/wy2009/pdfs/12020000.2009.pdf>

Washington Department of Transportation. 2009. *Annual Traffic Report*. Available at: <http://www.wsdot.wa.gov/mapsdata/tdo/annualtrafficreport.htm>

Washington Department of Transportation. September 2008. *Storm-Related Closures of I-5 and I-90: Freight transportation Economic Impact Assessment Report*.

Washington State Transportation Commission. December 2006. *Statewide Rail Capacity and System Needs Study. Task 10.2 – Washington State Rail Investment Plan*. Available at www.wstc.com.

Washington Trails Association. *Projected Park Closures*. Accessed September 24, 2010. Available online: <http://www.wta.org/trail-news/magazine/projected-state-park-closures>

Appendix A

**Table A-1
Building Clean-Up Costs Detail**

	10-Year	50-Year	100-Year	500-Year	<i>formula</i>
Base					
Residential Buildings, number	218	357	514	1461	<i>a</i>
Damaged Residential Floor space, sq ft	348,800	571,200	822,400	2,337,600	$b = a \times 1,600$
Commercial Buildings, number	1	12	13	25	<i>c</i>
Damaged Commercial Floor space, sq ft	15,800	189,600	205,400	395,000	$d = c \times 15,800$
Total Damage, \$	\$1,567,385	\$3,270,616	\$4,418,426	\$11,747,219	$e = (b + d) \times \$4.30$
With Upper Chehalis Structure					
Residential Buildings, number	181	288	330	706	<i>f</i>
Damaged Residential Floor space, sq ft	289,600	460,800	528,000	1,129,600	$g = f \times 1,600$
Commercial Buildings, number	0	4	8	13	<i>h</i>
Damaged Commercial Floor space, sq ft	0	63,200	126,400	205,400	$i = h \times 15,800$
Total Damage, \$	\$1,244,966	\$2,252,632	\$2,813,211	\$5,739,054	$j = (g + i) \times \$4.30$
Upper Chehalis Project Benefit	\$322,419	\$1,017,983	\$1,605,215	\$6,008,166	$k = e - j$
With South Fork Chehalis Structure					
Residential Buildings, number	202	326	417	1353	<i>l</i>
Damaged Residential Floor space, sq ft	323,200	521,600	667,200	2,164,800	$m = l \times 1,600$
Commercial Buildings, number	1	8	12	21	<i>n</i>
Damaged Commercial Floor space, sq ft	15,800	126,400	189,600	331,800	$o = n \times 15,800$
Total Damage, \$	\$1,457,333	\$2,785,698	\$3,683,312	\$10,732,675	$p = (m + o) \times \$4.30$
South Fork Chehalis Project Benefit	\$110,052	\$484,918	\$735,115	\$1,014,544	$q = e - p$
With Both Projects					
Residential Buildings, number	175	260	301	520	<i>r</i>
Damaged Residential Floor space, sq ft	280,000	416,000	481,600	832,000	$s = r \times 1,600$
Commercial Buildings, number	0	3	5	11	<i>t</i>
Damaged Commercial Floor space, sq ft	0	47,400	79,000	173,800	$u = t \times 15,800$
Total Damage, \$	\$1,203,697	\$1,992,118	\$2,409,973	\$4,323,850	$v = (s + u) \times \$4.30$
Benefit from Both Projects	\$363,688	\$1,278,498	\$2,008,454	\$7,423,369	$w = e - v$

